

GEOTECHNICAL INVESTIGATION, PRELIMINARY ACID SULFATE SOILS ASSESSMENT, SALINITY ASSESSMENT & WASTEWATER ASSESSMENT

FOR

AMJ DEMOLITION & EXCAVATION PTY LIMITED

55 Martin Road, Badgerys Creek, New South Wales

Report No: 17/3905A

Project No: 21649/8653C

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DRAWING NO. 17/3905 – BOREHOLE AND PENETROMETER LOCATIONS

DRAWING NO. 17/3905A – PROPOSED WASTEWATER DISPOSAL AREA

NOTES RELATING TO GEOTECHNICAL REPORTS

APPENDIX A – BOREHOLE LOGS AND EXPLANATION SHEETS

APPENDIX B – LABORATORY TEST RESULTS

APPENDIX C - BUREAU OF METEOROLOGY DATA

APPENDIX D - WATER BALANCE CALCULATIONS

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1. INTRODUCTION

This report presents the results of a combined Geotechnical Investigation, Preliminary Acid Sulfate Soils Assessment, Salinity Assessment and Wastewater Assessment carried out by STS GeoEnvironmental Pty Limited (STS) for a proposed new commercial development to be constructed at 55 Martin Road, Badgerys Creek. We have been informed the works comprise the construction of a waste resource recovery centre which will include the following:

- Construction of a large (22m x 70m) shed with a concrete floor,
- Construction of an office building and staff/visitor car park,
- Construction of an unsealed stockpile and vehicle movement area,
- Construction of a wheel wash and weighbridge,
- Installation of an on-site wastewater disposal system, and
- Construction of sedimentation basins

We understand that the pavement design is required to satisfy heavy goods vehicle movements. The purpose of the salinity assessment was to determine if the site is affected by levels of soil salinity that would require specific management intervention in line with Councils DA requirements. The purpose of the Preliminary Acid Sulfate Soils Assessment was to determine if the site is affected by actual or potential Acid Sulfate Soils that would require specific management intervention in line with Councils DA requirements.

The purpose of the investigation was to:

- assess the subsurface conditions over the site,
- provide a site classification to AS2870,
- provide recommendations regarding the appropriate foundation system for the site including design parameters,
- provide retaining wall design parameters,
- comment on safe batter slopes,
- comment on soil aggressiveness to buried steel and concrete,
- provide a pavement design for rigid, flexible and un-sealed pavements,
- comment on site preparation and re-grading,
- undertake a salinity assessment,
- undertake a wastewater assessment, determining the area required for using both surface and subsurface irrigation systems,
- undertake a Preliminary Acid Sulfate Soils Assessment.



In regards to the salinity assessment, the procedures given in the publication below, have been adopted for this study:

Reference 1: DLWC (2002) publication, "Site Investigation for Urban Salinity."

The wastewater assessment has been undertaken in accordance with the following publications:

Reference 2: AS/NZS 1547:2012, "On-site domestic wastewater management" Standards Australia.

Reference 3: Department of Local Government (1998), "On-site Sewerage Management for Single Households," Environment and Health Protection Guidelines.

The investigation was undertaken at the request of Claron Consulting Pty Limited on behalf of AMJ Demolition & Excavation Pty Limited.

Our scope of included a Preliminary Site Investigation (PSI) contamination assessment. The results of the PSI have been reported separately.

2. NATURE OF THE INVESTIGATION

2.1. Fieldwork

The fieldwork consisted of drilling nineteen (19) boreholes numbered BH1 to BH19, inclusive, and undertaking ten (10) Dynamic Cone Penetrometer (DCP) tests at the locations shown on Drawing No. 17/3905. The boreholes were drilled using a combination of Christie and Edson RP70 utility mounted drilling rigs owned and operated by STS. Soils and weathered rock were drilled using rotary solid flight augers. In order to monitor groundwater levels and obtain water samples, PVC standpipe piezometers was installed in BH2, BH8 and BH15.

Drilling operations were undertaken by STS's technical officers and senior geologists who also logged the subsurface conditions encountered and collected samples for testing purposes.

The subsurface conditions observed are recorded on the borehole logs given in Appendix A. An explanation of the terms used on the logs is also given in Appendix A. Notes relating to geotechnical reports are also attached.

All soil samples were collected directly from the augers using hand tools and were transferred directly into new clean jars or bottles prepared by Australian Laboratory Services (ALS). Water samples were collected using a disposable polyurethane bailer. All jars and bottles were filled to the rim to minimize head space. The samples were then placed into ice-filled chests and transferred to ALS for testing purposes. Chain of Custody documentation was used to record and track the samples.

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All sampling equipment was decontaminated prior to use and between sampling locations by washing with a mixture of water and DECON 90 and rinsing with potable water.

2.2. Laboratory Testing

In order to assess the soils for their aggressiveness, levels of salinity and to conduct the wastewater assessment, representative soil samples were tested to determine the following:

- Electrical Conductivity (EC),
- pH,
- Sulfate Content (SO4),
- Chloride Content (CI),
- Exchangeable Sodium Precent (ESP),
- Cation exchange capacity (CEC),
- Phosphorous Sorption Index,
- Emerson Class Number, and
- Particle Size Distribution.

In order to determine the pavement thickness, the California Bearing Ratio (CBR) of the pavement subgrade material was determined. The tests were carried out on samples compacted to a density ratio of 100% of the Standard maximum dry density.

Shrink swell testing was also undertaken to assist with determining the site classification.

The detailed test reports are given in Appendix B.

3. GEOLOGY AND SITE CONDITIONS

The Penrith geological series sheet at a scale of 1:100,000 shows Triassic Age Bringelly Shale of the Wianamatta Group underlies the site. Rocks within this formation comprise shale, claystone and laminite. Sandstone lenses are known to exist.

The site is rectangular in shape with an area of approximately 2.54ha. At the time of the fieldwork, the site comprised a rural residential parcel of land consisting of grassed paddocks with sparse trees and shrubs.

The north-east portion of the site comprises an enclosed area of about 2,900m² with 42m frontage to Martin Road. This part of the site is occupied by a single storey brick residence with gravel driveway, a metal shed and few mature trees. The remainder of the site is undeveloped.

A small dam with a footprint of about 40m^2 is located in the north-west corner of the site.

The ground surface falls to the west with a total fall of approximately 8 metres from RL 59.5m to RL 51.5m.

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The land to the north is vacant and undeveloped, whilst the land to the south is rural residential. To the east and west are Martin Road and Lawson Road respectively.

4. SUBSURFACE CONDITIONS

When assessing the subsurface conditions across a site from a limited number of boreholes there is the possibility that variations may occur between test locations. The data derived from the site investigation programme are extrapolated across the site to form a geological model and an engineering opinion is rendered about overall subsurface conditions and their likely behaviour regarding the proposed development. The actual conditions at the site may differ from those inferred, since no subsurface exploration programme, no matter how comprehensive, can reveal all subsurface details and anomalies.

The subsurface conditions generally consist of topsoil overlying silty clays, sandy clays and weathered sandstone and shale. Topsoil materials were encountered across the site in all boreholes to depths of 0.3 to 0.5 metres. Natural silty clays and sandy clays were encountered below the topsoil to depths of 1.3 to 3.6 metres. The consistency of the clays varies from firm to stiff to very stiff. Weathered shale and sandstone underlies the site to the depth of auger refusal, 3.2 to greater than 6.0 metres.

Groundwater seepage was not observed during auger drilling of the boreholes. Six days later the water levels in the piezometers were recorded at 2.05m below the existing ground surface level in BH2 and 2.6 metres in BH8. BH15 remained dry.

5. GEOTECHNICAL RECOMMENDATIONS

5.1. Site Classification to AS2870

Table 5.1 below presents the results of the shrink swell testing undertaken:

Table 5.1 – Shrink Swell Index Summary Table

Location	Depth	Material Description	Shrink Swell Index (ISS)
вн6	0.7 – 1.0	SILTY CLAY: Light brown with light grey (Natural)	1.9
BH7	0.6 – 0.9	SILTY CLAY: Light brown with light grey (Natural)	1.8
BH15	0.5 – 0.9	SILTY CLAY: Orange brown with light grey (Natural)	1.7

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The classification has been prepared in accordance with the guidelines set out in the "Residential Slabs and Footings" Code, AS2870 – 2011.

Because there are trees present, abnormal moisture conditions (AMC) prevail at the site (Refer to Section 1.3.3 of AS2870).

Because of the AMC, the site is classified *a problem site (P)*. Provided that the recommendations given below are adopted and the footings are founded in natural underlying any topsoil, the site may be reclassified *moderately reactive (M)*.

5.2. Foundation Design

Footings that bear in firm to stiff natural soils underlying any topsoil may be proportioned using an allowable bearing pressure of 100 kPa. This value may be increased to 150 kPa in stiff soils and 300 kPa in very stiff soils. The minimum depth of founding must comply with the requirements of AS2870. In order to overcome the presence of trees, the foundations are to be designed in accordance with Appendices H and CH of AS2870.

Should a higher bearing pressure be required, then the loads should be transferred using piles to underlying stronger materials. Piles founded in the very stiff natural soils may be proportioned using an allowable bearing pressure of 450 kPa, provided that the pier depth to diameter ratio exceeds a value of 4. An allowable adhesion of 20 kPa applies to the portion of the shaft within the natural soils below a depth of 0.5 metres.

Piles founded in weathered shale/sandstone may be proportioned using an allowable bearing pressure of 700 kPa. An allowable adhesion of 70 kPa may be adopted for the portion of the shaft within the weathered shale/sandstone. These values may be increased to 1000 kPa and 100 kPa, respectively, when founding below the depth of auger refusal as shown on the borehole logs. When piles are founded in rock the adhesion in the overlying soils must be ignored.

In order to ensure the bearing values given can be achieved, care should be taken to ensure that the base of excavations are free of all loose material prior to concreting. It is recommended that all footing excavations be protected with a layer of blinding concrete as soon as possible, preferably immediately after excavating, cleaning, inspection and approval. The possible presence of groundwater needs to be considered when drilling piers and pouring concrete.



5.3. Pavement Design and Construction

5.3.1 Concrete Pavement for Heavy Vehicle Movements

The laboratory testing carried out indicated the existing subgrade has a CBR value of 1.5%. The design traffic volume is difficult to determine for this type of development. In the absence of design traffic loadings, we have adopted a design traffic loading of 1×10^6 Commercial Vehicle Axle Group (CVAGs). Using the above data, the suggested pavement thickness is as follows:

Table 5.2 – Concrete Pavement Thickness Design

28 Day Concrete Strength (MPa)	Concrete Base Thickness (mm)	Subbase Thickness (mm)
32	190	100
40	160	100

The above thickness assumes that the pavement extends a minimum of 600mm beyond the edge of the trafficked lane/area.

5.3.2 Concrete Pavement for Car Park Area

The laboratory testing carried out indicated the existing subgrade has a CBR value of 1.5%. The design traffic volume is difficult to determine for this type of development. In the absence of design traffic loadings, we have adopted a design traffic loading of 5×10^4 Commercial Vehicle Axle Group (CVAGs). This allows for infrequent use of the car park for commercial vehicles, such as weekly garage collection. Using the above data, the suggested pavement thickness is as follows:

Table 5.3 – Concrete Pavement Thickness Design

28 Day Concrete Strength (MPa)	Concrete Base Thickness (mm)	Subbase Thickness (mm)
32	170	100
40	140	100

The above thickness assumes that the pavement extends a minimum of 600mm beyond the edge of the trafficked lane/area.



5.3.3 Flexible Pavement for Heavy Vehicle Movements

The flexible pavement thicknesses have been determined using the procedures given in Australian Roads Research Board (ARRB) "Sealed Local Roads Manual." We have assumed a 95% confidence level that the pavement will perform satisfactorily during its design life. A design traffic loading of 1×10^6 ESAs is considered appropriate for the site. For a subgrade CBR value of 1.5%, the suggested pavement thickness is a recommended minimum of 610 mm, made up as follows:

Table 5.4 – Flexible Pavement Thickness Design

Material Type	Minimum Thickness (mm)
AC	50
Base Course	150
Subbase	410
TOTAL	610

Due to the low CBR value recorded, the above thickness assumes that he subgrade will be stabilised with 2% lime to a depth of 150mm.

The Asphaltic Concrete (AC) layer has been included as a wearing course, and has not been considered as providing structural capacity to the pavement. If an unsealed pavement is required, then the AC layer may be omitted.

5.3.4 Flexible Pavement for Car Park Area

The flexible pavement thicknesses have been determined using the procedures given in Australian Roads Research Board (ARRB) "Sealed Local Roads Manual." We have assumed a 95% confidence level that the pavement will perform satisfactorily during its design life. A design traffic loading of 6×10^4 ESAs is considered appropriate for the site. This allows for infrequent use of the car park for commercial vehicles, such as weekly garage collection For a subgrade CBR value of 1.5%, the suggested pavement thickness is a recommended minimum of 530 mm, made up as follows:

Table 5.5 – Flexible Pavement Thickness Design

Material Type	Minimum Thickness (mm)
AC	50
Base Course	100
Subbase	380
TOTAL	530

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Due to the low CBR value recorded, the above thickness assumes that he subgrade will be stabilised with 2% lime to a depth of 150mm.

5.3.5 Construction

The designs given above assume adequate provisions have been made for both surface and subsurface water.

The clayey site soils, which will make up the pavement subgrade are reactive. They will therefore be susceptible to shrinkage and swelling due to moisture content changes. If these subgrade soils are allowed to dry following compaction, it is probably that shrinkage will occur resulting in cracking. After placement of the pavement materials, the subgrade soils will moisten, resulting in swelling and partial loss of strength. It is therefore recommended that the subgrade be covered as soon as possible after completion of compaction in order to minimise the potential for evaporation and shrinkage to occur.

The subgrade materials should be compacted to a minimum density ratio of 100% of the Standard maximum dry density. Compaction should be verified by proof rolling and in-situ density tests. Base and subbase course materials should be compacted and tested to a minimum density ratio of 98% of the Modified maximum dry density. The level of compaction should be verified by in-situ density testing.

All pavement materials used should comply with the local council requirements.

5.4. Safe Batter Slopes

In the short term, dry cut slopes should remain stable at an angle of 1 to 1. In the long term dry cut slopes formed at an angle of 2(H) to 1(V) should remain stable. Slopes cut at this angle would be subject to erosion unless protected by topsoil and diversion drains at the crest of the slopes. In order to use mowers to maintain cut slopes, an angle of 4(H) to 1(V) or flatter should be used.

5.5. **Retaining Wall Design**

The parameters used to proportion the retaining walls depend on whether the walls can be permitted to deflect. For walls, which cannot be permitted to deflect, the "at rest" (K_o) conditions should be adopted. A value of 0.6 should be adopted. For walls that can be allowed to deflect, an active earth pressure coefficient (Ka) of 0.4 should be adopted. A passive earth pressure coefficient (K_p) of 2.5 may be used for the clays. A bulk density of 20 kN/m³ may be used.

As with all retaining walls, the above coefficient must be adjusted for ground surface slope, groundwater and external loads, such as buildings and vehicles.

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5.6. Site Preparation and Re-Grading

The performance of the slabs and pavements cannot be guaranteed unless the following procedures are adopted during the site earthworks:

- Remove any vegetation, topsoil and fill present. The exposed subgrade should be inspected by a geotechnical engineer who may wish to proof roll the exposed subgrade with a heavy, non-vibrating roller to detect soft or wet areas. These areas should be excavated to competent material and then filled as detailed below.
- Fill the site to the underside of slab or pavement level, in layers not exceeding 200 mm loose thickness, compacted to achieve a density ratio in the range of 98% to 102% of the Standard maximum dry density, at a moisture content within the range of -2% to +2% of the optimum for the material adopted.

The onsite silty clays can become untrafficable during periods of wet weather.

5.7. Soil Aggressiveness

The aggressiveness or erosion potential of an environment in building materials, particularly concrete and steel is dependent on the levels of soil pH and the types of salts present, generally sulphates. In order to determine the degree of aggressiveness, the test values obtained are compared to Tables 6.4.2 (C) and 6.5.2 (C) in AS2159 – 2009 Piling – Design and Installation and Tables 5.1 and 5.2 of AS2870-2011. In regards to the electrical conductivity, the laboratory test results have been multiplied by the appropriate factor to convert the results to EC_e. The test results are summarised in Table 5.6 below.

Table 5.6 – Soil Aggressiveness Summary Table

Sample No.	Location	Depth (m)	рН	Sulfate (mg/kg)	Electrical Conductivity (dS/m)	
					EC _{1:5}	ECe
S2-3	BH2	1.0	5.2	180	0.640	4.480
S2-5	BH2	2.0	5.1	80	0.741	5.187
S2-6	BH2	2.5	5.4	160	0.790	5.530
S2-8	BH2	4.0	7.2	100	0.693	6.237
S8-2	BH8	0.5	6.2	10	0.155	1.085
S8-4	BH8	1.5	8.7	140	1.120	7.840
S8-5	BH8	2.0	8.7	120	0.944	6.608
S8-7	BH8	3.0	9.1	110	0.736	6.624
S15-2	BH15	0.5	6.4	90	0.112	0.784
S15-3	BH15	1.0	8.8	120	0.446	3.122

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Table 5.6 – Soil Aggressiveness Summary Table (Cont.)

Sample No.	Location	Depth (m)	рН	Sulfate (mg/kg)	Electrical Conductivity (dS/m)	
					EC _{1:5}	ECe
S15-5	BH15	2.0	8.7	10	0.192	1.728
S15-6	BH15	2.5	8.6	20	0.224	2.016

The report results range between:

• pH - 5.1 to 9.1

soluble SO₄ - 10 to 180 mg/kg (ppm)
 EC_e - 0.784 to 7.840 dS/m

The soils on the site consist of low permeability silty clays. Therefore, the soil conditions B are considered appropriate.

A review of the durability aspects indicates that:

• pH : minimum value of 5.1

• SO₄ : maximum value of 180 mg/kg (ppm) < 5000 ppm

EC_e : maximum value of 7.8 dS/m

The exposure classification for the onsite soils is non-aggressive for steel and mildly aggressive to concrete in accordance with AS2159-2009. The soils are classified as A2 in accordance with AS2870-2011.

6. SALINITY ASSESSMENT

6.1. Soil Test Results

The results of the soil sample analyses are provided in Tables 6.1 to Table 6.3. Table 6.1 also includes the appropriate multiplier factors used to convert results to EC_e ($\mu S/cm$) and the salinity class with which the soil sample falls according to Table 6.2: EC_e Values of Soil Salinity Classes in the publication entitled "Site Investigation for Urban Salinity (DLWC, 2002)".



Table 6.1 – Salinity Results

Sample ID	Sample Depth (m)	EC _{1:5} (μS/cm)	Soil Type	Multiplier Factor	EC _e (μS/cm)	Salinity Class
S1-1	0.2	724	Silty Clay	7	5068	Moderately Saline
S2-2	0.5	437	Silty Clay	7	3059	Slightly Saline
S2-3	1.0	640	Silty Clay	7	4480	Moderately Saline
S2-4	1.5	780	Silty Clay	7	5460	Moderately Saline
S2-5	2.0	741	Silty Clay	7	5187	Moderately Saline
S2-6	2.5	790	Silty Clay	7	5530	Moderately Saline
S2-7	3.0	723	Silty Clay	7	5061	Moderately Saline
S2-8	4.0	693	Shale	9	6237	Moderately Saline
S4-1	0.2	226	Silty Clay	7	1582	Non Saline
S6-1	0.2	52	Silty Clay	7	364	Non Saline
S7-1	0.2	84	Silty Clay	7	588	Non Saline
S8-1	0.2	76	Silty Clay	7	532	Non Saline
S8-2	0.5	155	Silty Clay	7	1085	Non Saline
S8-3	1.0	997	Silty Clay	7	6979	Moderately Saline
S8-4	1.5	1120	Silty Clay	7	7840	Moderately Saline
S8-5	2.0	944	Silty Clay	7	6608	Moderately Saline
S8-6	2.5	666	Shale	9	5994	Moderately Saline
S8-7	3.0	736	Shale	9	6624	Moderately Saline
S8-8	4.0	570	Shale	9	5130	Moderately Saline
S9-1	0.2	430	Silty Clay	7	3010	Slightly Saline
S11-1	0.2	155	Silty Clay	7	1085	Non Saline
S12-1	0.2	87	Silty Clay	7	609	Non Saline
S13-1	0.2	58	Silty Clay	7	406	Non Saline
S14-1	0.2	100	Silty Clay	7	700	Non Saline
S15-1	0.2	87	Silty Clay	7	609	Non Saline
S15-2	0.5	112	Silty Clay	7	784	Non Saline
S15-3	1.0	446	Silty Clay	7	3122	Slightly Saline
S15-4	1.5	350	Sandstone	9	3150	Slightly Saline
S15-5	2.0	192	Sandstone	9	1728	Non Saline
S15-6	2.5	224	Sandstone	9	2016	Slightly Saline
S15-7	3.0	240	Sandstone	9	2160	Slightly Saline
S15-8	4.0	337	Sandstone	9	3033	Slightly Saline
S17-1	0.2	37	Silty Clay	7	259	Non Saline
S19-1	0.2	46	Silty Clay	7	322	Non Saline



Table 6.2 –Summary of ESP Results

Sample No.	Location	Depth (m)	ESP (%)	Sodicity
S2-3	BH2	1.0	21.5	Highly Sodic
S2-5	BH2	2.0	30.4	Highly Sodic
S2-6	BH2	2.5	29.9	Highly Sodic
S2-8	BH2	4.0	24.6	Highly Sodic
S8-2	BH8	0.5	10.3	Sodic
S8-4	BH8	1.5	6.6	Sodic
S8-5	BH8	2.0	13.4	Sodic
S8-7	BH8	3.0	9.0	Sodic
S13-1	BH13	0.2	1.4	Non-Sodic
S15-2	BH15	0.5	12.9	Sodic
S15-3	BH15	1.0	8.2	Sodic
S15-5	BH15	2.0	<0.2	Non-Sodic
S15-6	BH15	2.5	<0.2	Non-Sodic

Table 6.3 –Summary of Emerson Class Number Results

Sample No.	Location	Depth (m)	Emerson Class No.	Classification
8653/C1	BH2	0.5 - 1.1	6	Slaking, no dispersion
8653/C2	BH4	1.0 - 1.4	5	Slaking, no dispersion
8653/C3	вн8	0.3 – 0.8	3	Slaking, dispersion after remoulding
8653/C4	BH17	0.4 – 1.0	3	Slaking, dispersion after remoulding

 EC_e is representative of the actual salinity level that the plant roots are exposed to and as such provides an indication of the toxicity of the soils to various plant species. Reported EC_e for the samples ranged from 259 μ S/cm to 7840 μ S/cm and may be classified as non-saline to moderately saline.

Sodicity is expressed as the amount of exchangeable sodium as a percentage of the Cation Exchange Capacity or ESP %. Soil with an ESP of less than 5% is considered non-sodic. Those with an ESP between 5 and 15% are considered sodic whereas those with an ESP greater than 15% are considered highly sodic. The ESP results indicate that the on-site soils which overly shale bedrock are sodic to highly sodic, whereas the soils which overly sandstone bedrock are non-sodic to sodic.

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The results of the Emerson Class Number testing indicate that the on-site soils are Class 3 to Class 6. Soils of Class 3 are slaking and no dispersion before remoulding, dispersion after remoulding. Soils of Class 5 are slaking and no dispersion before remoulding, no dispersion after remoulding, no calcite or gypsum present and dispersion after slaking in a 1:5 soil/water suspension. Soils of Class 6 are as per Class 5, however experience flocculation after slaking in a 1:5 soil/water suspension. These results indicate that the soils are mostly non-dispersive.

6.2. Groundwater Salinity

As noted above, standpipe piezometers were installed in borehole BH2, BH8 and BH15. After installation, the piezometer was dewatered prior to sampling. Water samples were obtained six days later to ensure the sample was representative of the in-situ conditions. A description of salinity in water has been developed by Australia Water Resources Council and is given in Table 6.3.

Table 6.3 – Class of Groundwater Salinity

Class	Electrical Conductivity (μS/cm)
Fresh	0 – 800
Marginal	800 – 1600
Brackish	1600 – 4800
Saline	>4800

The electrical conductivity measured in SAL1 (BH8) is 35500 μ S/cm, the electrical conductivity measured in SAL2 (BH2) is 33000 μ S/cm. BH15 remained dry. This indicates the groundwater can be classified as saline.

6.3. Potential Impacts on Development

The general impacts that have the potential to occur may be summarised as:

- Damage to and subsequent reduction of the lifespan of buildings and associated infrastructure such as roads and underground services as a result of construction close to aggressive soil and groundwater. This may include:
 - Degradation of bricks, concrete, road base and curbing materials leading to expansion, cracking, strength and mass loss;



- Corrosion of reinforcement and loss of structural integrity;
- o Rising/falling damp; and
- Non-structural impacts, such as efflorescence on bricks.
- Degradation of drainage infrastructure by a rise in the groundwater level. Damage to pipes
 has the potential to exacerbate the problem by further recharging the shallow
 groundwater; and
- Damage to or prevention of the cultivation of salt-sensitive vegetation in landscaped areas and gardens may arise across the site due to the salinity levels in surface soils.

The risks considered to be potentially posed to individual assets and activities and appropriate management options are detailed below.

The construction and maintenance stages of the proposed development have the potential to adversely affect salinity conditions on the site and in the surrounding area, mostly by altering the current hydrological cycle. Potential impacts include:

- A rise in the groundwater level due to increased water inputs associated with urban development. e.g. irrigation and leaking pipes. Reduced infiltration due to the construction of hardstand across the site may offset this to some extent;
- Altered flow and drainage patterns which may result in increased water accumulation and associated salinity issues in areas of impeded flow, as a consequence of e.g. the construction of drainage lines, footings and roads;
- Interception of groundwater should local groundwater levels be raised by prolonged periods of precipitation, creation of a perched water table, or increased recharge of the regional or localized aquifer may result from cutting or compaction within the perched or permanent aquifer;
- Excavation and redistribution of saline soil during excavation and filling operations around the site.

These impacts have the potential to lead to an increase in the surface expression of soil salinity and adversely affect downstream water quality.



6.4. Salinity Model

The testing results (provided in Table 6.1 to 6.3) indicates that the soils tested are classed as being mostly non saline to moderately saline. The majority of the near surface soils were non saline. Therefore, the soils are unlikely to present a risk of producing adverse salinity-based impacts. The groundwater below the site is saline and occurs at depths of approximately 2.0 to 2.5 metres below the land surface. Further, the results suggest that the soils on site are classed as sodic to highly sodic and non-dispersive. Sodic soils have the potential to lose structure and become dispersive when saturated, and therefore can be both poorly draining and susceptible to erosion. However, many Australian soils are sodic and sodicity is not necessarily a function of land salinity.

Therefore, the main mechanisms by which salts could potentially be mobilised, thereby amplifying salinity issues, include;

- raising of the groundwater table;
- impedance of groundwater flow or surface water drainage;

These mechanisms would result in an increased surface expression of salinity.

6.5. Salinity Risk Assessment and Conclusions

Based on the results of the salinity assessment, the following conclusions are made:

- Soil salinity is not expected to impact on the proposed site development, therefore a salinity management plan will not be required.
- The groundwater beneath the site should not be extracted for use as an irrigation source;
- Standard landscaping procedures for urban development sites would be sufficient to prevent any surface expression of salinity or impacts due to sodic soils. Such procedures would include the design and installation of appropriate drainage, covering landscaping zones in topsoil and revegetating.
- Selection of appropriate building designs and materials would also be necessary to ensure
 that the integrity of building foundations and floor slabs is not compromised due to the
 natural acidity, electrical conductivity and concentrations of key anions in the soils.
 Reference should be made to Section 5.7 of this report for advice regarding the
 aggressiveness of soils to buried steel and concrete.



7. WASTEWATER ASSESSMENT

7.1. Introduction

Climate data used to prepare the wastewater management plan for the site is that recorded by the Australian Government Bureau of Meteorology at Prospect Water Reservoir, Prospect, about 12.0km north east of the site. Details are given in Appendix A.

Table 7.1 – Monthly Rainfall and Evaporation Data

Month	Rainfall (Median) (mm)	Average Evaporation (mm)
January	73.2	170.5
February	73.1	131.6
March	78.3	120.9
April	57.2	87
May	38.4	62
June	50.0	48
July	32.9	52.7
August	30.9	77.5
September	40.2	108
October	43.1	136.4
November	60.1	150
December	58.0	173.6

Note: Data was obtained from the Prospect Water Reservoir (Prospect) weather station via the Bureau of Meteorology.

7.2. Laboratory Test Results

The physical soil parameters are summarised in Table 7.2 and the chemical parameters in Table 7.3.

Table 7.2 – Physical Soil Properties

Location	Depth (m)	Clay (%)	Silt (%)	Sand (%)	Gravel (%)	Material Description ¹
BH13	0.0 - 0.4	17	16	44	23	Loam
BH14	0.0 - 0.4	14	20	48	18	Loam

¹ = As given in AS/NZS 1547:2012



Table 7.3 – Soil Chemical Properties

Location	Depth (m)	рН	Electrical Conductivity (µS/cm)	CEC (meq/100g)	ESP (%)	Phosphorous Sorption Capacity (mgP)
BH13	0.2	6.7	58	17.1	1.4	766
BH14	0.9	6.9	100	17.7	11.5	1090

Based on the results in Table 2, the Design Irrigation Rate (DIR) has been determined using Table M1 in AS/NZ1547:2012. A DIR value of 21 mm/week (28 divided by a factor of safety of 1.3) has been adopted for a spray irrigation system.

7.3. Wastewater Assessment

Individual soil features are discussed below and a limitation rating is provided for each feature.

- Depth of soil greater than the 0.4 m minimum required.
- Depth to water table 2.0m.
- Soil permeability DIR values of 21 mm/week are consistent with a soil of moderate permeability. This poses a minor limitation.
- Emerson Crumb The soils are primarily Class 3, Class 5 and Class 6. These soils pose no limitation due to the soils potential to disperse.
- pH The values of 6.7 and 6.9 pose no limitation.
- Electrical conductivity this is a measure of soil salinity. Values below 4 dS/m (4000μ S/cm) pose no limitations. The measured values are significantly less than this value.
- Sodicity Exchangeable sodium percentage (ESP) is a measure of sodicity. Values less than 5 are considered non-sodic, whilst values greater than 15 are considered highly sodic. Values of 1.4 and 11.5 indicate non sodic to sodic soils are present. This poses a minor limitation.
- Cation Exchange Capacity (CEC) A measure of the soil's ability to retain nutrients. Values in excess of 15 meq/100g pose no constraints. The measures values of 17.1 and 17.7 are in excess of 15 and therefore pose no limitation.



Phosphorus Sorption – A measure of the soil's ability to immobilise excess phosphorus. Values
in excess of 6000 kg/ha pose no constraints. Values of less than 2000 kg/ha pose a major
limitation. The measured values are less than 2000kg/ha, and therefore pose a major
limitation.

The above assessment indicates there are minor and major limitations on the soils

7.4. Site Constraints

Individual site features are discussed below and a limitation rating provided for each:

- Flood potential It is unknown whether the site is above the 1 in 100-year flood contour.
- Exposure The proposed disposal area has good wind and sun exposure.
- Slope The slopes on the site are less than 5 degrees. Ensuring a good grass cover is maintained in the spray areas should ensure minimal if any erosion.
- Run on and up slope drainage Where this is excessive, wastewater can be transported off site. The site has a gentle slope so run on drainage should not pose a limitation.
- Erosion potential None visible on the site.
- Site drainage No sign of surface dampness.
- Rock outcrops None present on the site.
- Fill No fill is present.
- Geology There are no geological discontinuities in the area.
- Buffer distances The buffer distances given in Table 7.4 should be adopted.



Table 7.4 – Recommended Buffer Distances

System	Recommended Buffer Distances
All land	100 metres to permanent surface waters (e.g. river, streams, lakes, etc)
application	250 metres to domestic groundwater well
systems	40 metres to other waters (e.g. farm dams, intermittent waterways and
	drainage channels, etc)
Surface	6 metres if area up-gradient and 3 metres if area down-gradient of driveways
spray	and property boundaries
irrigation	15 metres to dwellings
	3 metres to paths and walkways
Subsurface	6 metres if area up-gradient and 3 metres if area down-gradient of swimming
irrigation	pools, property boundaries, driveways and buildings.

7.5. Required Irrigation Area

The design criteria for sizing the required wastewater irrigation area are detailed in AS 1547. The required area for spray irrigation is calculated as follows:

$$A_i = q_w/DIR$$

Where

 A_i = irrigation area required (m²)

qw = total quantity of effluent generated per week (L-litres)

DIR = design irrigation rate (litres/m²/week)

The Australian Standard estimates a minimum design daily effluent flow of 180 litres per person per day for occupants. This assumes the office will be fitted with two toilets with hand basins, together with a kitchen area with sink. We have assumed the office will be accommodate up to 6 occupants. This equates to 7560 litres of weekly effluent.

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For a DIR value of 21 litres/week, the minimum surface irrigation area required is:

$$A_i = 6300/21 = 360 \text{ m}^2$$



7.6. Hydraulic Loading

The hydraulic loading provides an indication of the potential periods when wet weather storage may be required. The hydraulic loading is given by the following relationships:

Hydraulic Loading = Precipitation - (Evapo transpiration + Percolation)

The monthly hydraulic loadings for the sites are determined from the water balance given in Appendix D. Hydraulic loads in excess of zero indicate wastewater storage is required. A minimum subsurface spray irrigation disposal area of 475 m² will be required if no storage is provided for 6 people.

7.7. Nutrient Balance

The amount of nutrient available can be determined by multiplying the effluent application note by the amount of nutrient in the effluent. The available nutrients are given below in Table 7.5.

Table 7.5 – Available Nutrients

Effluent Rate	Nitrogen¹	Phosphorous ²
(litres per day)	(kg/yr)	(kg/yr)
1080	9.9	3.9

¹ = Assume a nominal rate of 30 mg/litre

In regards to the nitrogen, a nominal rate of 25 mg/m^2 /day has been assumed for the uptake of nitrogen into the soil. We have assumed that 50% of the nitrogen will be either lost to the atmosphere or taken up by the vegetation.

The area required is calculated as follows:

A =
$$(0.5 \times 30) \times Q$$
 Q = flow rate (L/d)
25
= $648m^2$

² = Assume nominal rate of 12 mg/litre



The phosphorous sorption capacity of the onsite soils range between 766 and 1,090 mg/kg, with an average value of 928 mg/kg. Based on a bulk unit weight of 17kN/m³ and an effective thickness of 0.4m, this equates to an uptake of 0.63 kg/m².

The area required for a 50-year life can be determined by multiplying the life required by the available phosphorous. This equates to

Area = P $\frac{\text{generated}}{\text{P uptake}}$ = $\frac{50 \times 3.9}{0.63}$ = $\frac{310 \text{ m}^2}{0.63}$

7.8. Conclusion

Based on the above assessment the required area for the different criteria are given below in Table 7.6:

Table 7.6 – Summary Table

Criteria	Hydraulic	Nitrogen	Phosphorous
Area required (m ²)	475	648	310

The nitrogen requirements dictate the minimum disposal area required; i.e. 648 m².

The limitations associated with permeability have been addressed in the calculations given above.

A suitable disposal area has been indicated on Drawing No 17/3905A which is attached to this report. The proposed disposal area has taken into consideration the buffer distances given in Table 7.4 of this report.

8. PRELIMINARY ACID SULFATE SOILS ASSESSMENT

8.1. Introduction

ASS are the common name given to sediments and soils containing iron sulfides which, when exposed to oxygen generate sulfuric acid. Natural processes formed the majority of acid sulfate sediments when certain conditions existed in the Holocene geological period (the last 10,000 years).



Formation conditions require the presence of iron-rich sediments, sulfate (usually from seawater), removal of reaction products such as bicarbonate, the presence of sulfate reducing bacteria and a plentiful supply of organic matter. It should be noted that these conditions exist in mangroves, salt marsh vegetation or tidal areas, and at the bottom of coastal rivers and lakes.

The relatively specific conditions under which acid sulfate soils are formed usually limit their occurrence to low lying parts of coastal floodplains, rivers and creeks. This includes areas with saline or brackish water such as deltas, coastal flats, backswamps and seasonal or permanent freshwater swamps that were formerly brackish. Due to flooding and stormwater erosion, these sulfidic sediments may continue to be re-distributed through the sands and sediments of the estuarine floodplain region. Sulfidic sediment may be found at any depth in suitable coastal sediments – usually beneath the water table.

Any lowering in the water table that covers and protects potential ASS will result in their aeration and the exposure of iron sulfide sediments to oxygen. The lowering in the water table can occur naturally due to seasonal fluctuations and drought or any human intervention, when carrying out any excavations during site development. Potential ASS can also be the exposed to air during physical disturbance with the material at the disturbance face, as well as the extracted material, both potentially being oxidised. The oxidation of iron sulfide sediments in potential ASS results in ASS soils.

Successful management of areas with ASS is possible but must take into account the specific nature of the site and the environmental consequences of development. While it is preferable that sites exhibiting acid sulfate characteristics not be disturbed, management techniques have been devised to minimise and manage impacts in certain circumstances.

When works involving the disturbance of soil or the change of groundwater levels are proposed in coastal areas, a preliminary assessment should be undertaken to determine whether acid sulfate soils are present and if the proposed works are likely to disturb these soils.

8.2. Presence of ASS

Reference to the Liverpool ASS Risk Map indicates the property is within an area where there are no known occurrences of ASS. It should be noted that maps are a guide only.

The following geomorphic or site criteria are normally used to determine if acid sulfate soils are likely to be present:

Page 24

- sediments of recent geological age (Holocene)
- soil horizons less than 5 in AHD



- marine or estuarine sediments and tidal lakes
- in coastal wetlands or back swamp areas

8.3. Assessment

The property is at an elevation of about RL50 m AHD and is underlain by Bringelly Shale. This is not consistent with the geomorphic criteria necessary for the presence of ASS. Based on our onsite observations and the subsurface conditions exposed in the boreholes, it is our opinion that the proposed construction will not intercept any ASS. Based on the observations undertaken in the piezometers, it appears that any seepage into any excavations would be minor and as a consequence, construction will not result in the lowering of any groundwater that may be present in the area.

Our assessment is the proposed construction will not require the preparation of an Acid Sulfate Soil Management Plan

9. FINAL COMMENTS

During construction, should the subsurface conditions vary from those inferred above, we would be contacted to determine if any changes should be made to our recommendations.

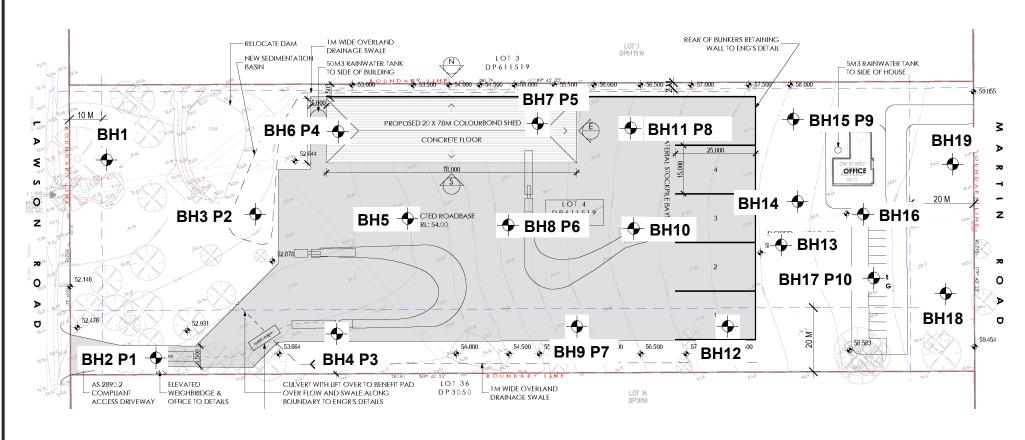
The exposed bearing surfaces for footings should be inspected by a geotechnical engineer to ensure the allowable pressure given has been achieved.

Matt Green

Senior Engineering Geologist

Laurie Ihnativ

Principal Geotechnical Engineer





STS GEOENVIRONMENTAL Pty. Ltd.

Scale: Unknown

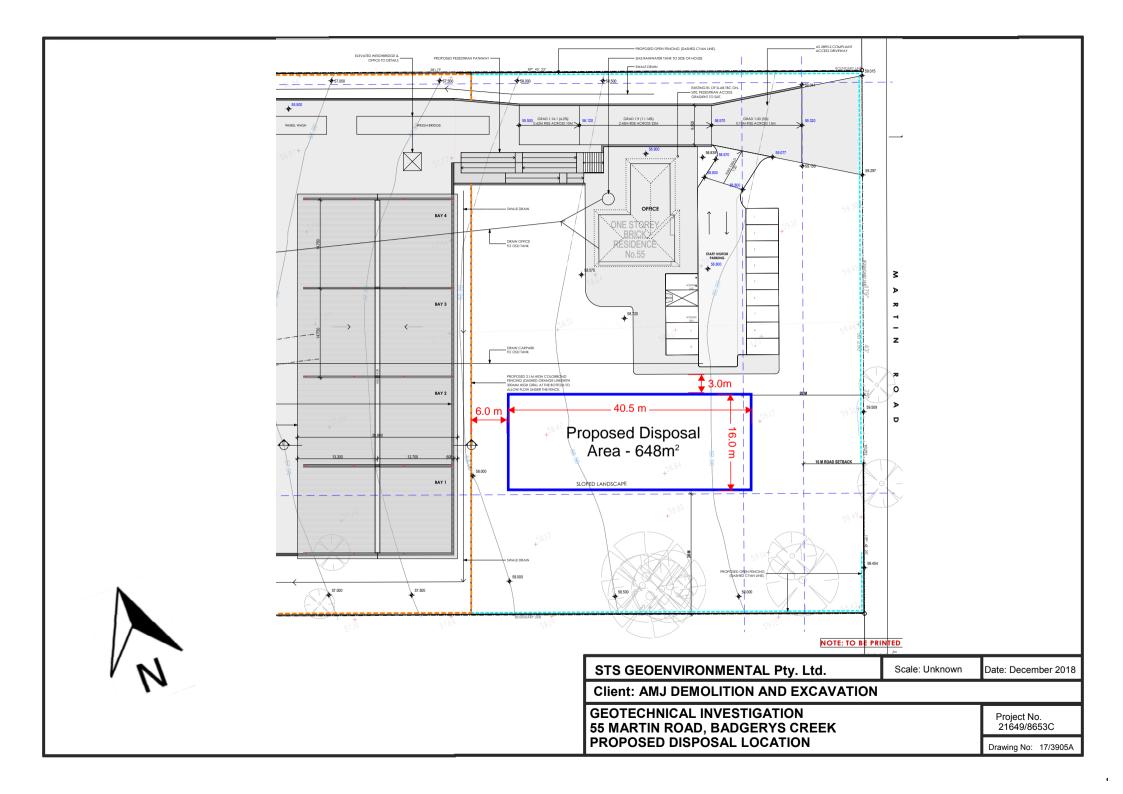
Date: December 2017

Client: AMJ DEMOLITION AND EXCAVATION

GEOTECHNICAL INVESTIGATION
55 MARTIN ROAD, BADGERYS CREEK
BOREHOLE AND PENETROMETER LOCATIONS

Project No. 21649/8653C

Drawing No: 17/3905



NOTES RELATING TO GEOTECHNICAL REPORTS

Introduction

These notes have been provided to outline the methodology and limitations inherent in geotechnical reporting. The issues discussed are not relevant to all reports and further advice should be sought if there are any queries regarding any advice or report.

When copies of reports are made, they should be reproduced in full.

Geotechnical Reports

Geotechnical reports are prepared by qualified personnel on the information supplied or obtained and are based on current engineering standards of interpretation and analysis.

Information may be gained from limited subsurface testing, surface observations, previous work and is supplemented by knowledge of the local geology and experience of the range of properties that may be exhibited by the materials present. For this reason, geotechnical reports should be regarded as interpretative rather than factual documents, limited to some extent by the scope of information on which they rely.

Where the report has been prepared for a specific purpose (eg. design of a three-storey building), the information and interpretation may not be appropriate if the design is changed (eg. a twenty storey building). In such cases, the report and the sufficiency of the existing work should be reviewed by SMEC Testing Services Pty Limited in the light of the new proposal.

Every care is taken with the report content, however, it is not always possible to anticipate or assume responsibility for the following conditions:

- Unexpected variations in ground conditions.
 The potential for this depends on the amount of investigative work undertaken.
- Changes in policy or interpretation by statutory authorities.
- The actions of contractors responding to commercial pressures.

If these occur, SMEC Testing Services Pty Limited would be pleased to resolve the matter through further investigation, analysis or advice.

Unforeseen Conditions

Should conditions encountered on site differ markedly from those anticipated from the information contained in the report, SMEC Testing Services Pty Limited should be notified immediately. Early identification of site anomalies generally results in any problems being more readily resolved and allows reinterpretation and assessment of the implications for future work.

Subsurface Information

Logs of a borehole, recovered core, test pit, excavated face or cone penetration test are an engineering and/or geological interpretation of the subsurface conditions. The reliability of the logged information depends on drilling/testing method, sampling and/or observation spacings and the ground conditions. It is not always possible or economic to obtain continuous high quality data. It should also be recognised that the volume or material observed or tested is only a fraction of the total subsurface profile.

Interpretation of subsurface information and application to design and construction must take into consideration the spacing of the test locations, the frequency of observations and testing, and the possibility that geological boundaries may vary between observation points.

Groundwater observations and measurements outside of specially designed and constructed piezometers should be treated with care for the following reasons:

- In low permeability soils groundwater may not seep into an excavation or bore in the short time it is left open.
- A localised perched water table may not represent the true water table.
- Groundwater levels vary according to rainfall events or season.
- Some drilling and testing procedures mask or prevent groundwater inflow.

The installation of piezometers and long term monitoring of groundwater levels may be required to adequately identify groundwater conditions.

Supply of Geotechnical Information or Tendering Purposes

It is recommended tenderers are provided with as much geological and geotechnical information that is available and that where there are uncertainties regarding the ground conditions, prospective tenders should be provided with comments discussing the range of likely conditions in addition to the investigation data.



APPENDIX A – BOREHOLE LOGS AND EXPLANATION SHEETS

Project:	55 Martin Ro	n and Excavationad, Badgerys C		Project / STS No.: 2 Date: December 12 Logged: DL		ВО	Sheet 1 of 1	BH 1
W A T T A E B R L E	S A M P L E S	DEPTH (m)	DESCRIPTION OF (Soil type, colour, grain size, plast	DRILLED PRODUC	т	S Y M B O L	CONSISTENCY (cohesive soils) or RELATIVE DENSITY (sands and gravels)	M O I S T U R E
	S1/1 @ 0.2 m		TY CLAY: brown with dark brown, low to med	ium plasticity, trace of	gravel TOPSOIL	CL		D-M
		1.0	REHOLE DISCONTINUED AT 0.3 M					
	D - disturbe WT - level o	d sample of water table or	U - undisturbed tube sample se water	B - bulk sample N - Standard Penetr	ration Test (SPT)	Contractor	:: STS t: Christie	
NOTES:	S - jar samp	le	ee explanation sheets for meaning of all descript	ve terms and symbols		Angle from	neter (mm): 100/200/30 a Vertical (°): V/Spiral/Two Prong	0

Project: 55 Ma Location: Refer t		ad Badgerys (n 1			
Location: Refer t	to Draw				Sheet 1 of 1	
	to Diaw	/ilig 140. 17/39	Logged, JK Checked By, WO			
W S				s	(cohesive soils) or	M O I
T A M	I			Y	RELATIVE	S
E B P R L L			DESCRIPTION OF DRILLED PRODUCT	M B	DENSITY (sands and	T U
E E S		DEPTH (m)	(Soil type, colour, grain size, plasticity, minor components, observations)	O L	gravels)	R E
S2-1/DU @ 0.2		_	SILTY CLAY: dark brown, medium plasticity	CL	FIRM TO STIFF	D
S2-2	-2					
@ 0.5	.5 m		TOPSOIL SILTY CLAY: red brown with orange brown and light grey, medium to high plasticity	CL/CH	STIFF	D-M
U50						
0.5-0.						
S2-3 @ 1.0		1.0				
В						M
@ 0.5-	1.1 m					
S2	-4		SILTY CLAY: light grey with yellow brown/orange brown, medium to high plasticity	CL/CH	VERY STIFF	M
@ 1.:						
WT		2.0				
18/12/17						
		-				
S2- @ 2.5						
2	.5 111					
		_				M-D
S2-7 @ 3.0		3.0				
w 5.0	.o m	3.0	WEATHERED SHALE: dark grey with light grey, clay seams, trace of fine grained sand		EXTREMELY LOW	D
		-			STRENGTH	
S2-3 @ 4.0		4.0				
4.0	.o iii	4.0				
		-				
		_				
		5.0				
		5.0				
		=	STANDPIPE PIEZOMETER INSTALLED			D-M
			DODELIOLE DISCONTINUED AT 6.0 M ON WEATHERED SHALE			
D - distr	turbed sa		BOREHOLE DISCONTINUED AT 6.0 M ON WEATHERED SHALE U - undisturbed tube sample B - bulk sample	Contractor	: STS	<u> </u>
II		vater table or f			: Edson RP70	
S - jar s	sample		1	Hole Diam	eter (mm): 100	
NOTES:	_		See explanation sheets for meaning of all descriptive terms and symbols	angle from	Vertical (°):	
				Drill Bit: S	Spiral	

Project:	MJ Demolitio 55 Martin Ro Refer to Drav	ad, Bad	gerys C	reek Date: December 12, 2017	В	OREHOLE NO.: Sheet 1 of 1	ВН 3
W A T T A E B R L E	S A M P L E S	DEP (m	тн	DESCRIPTION OF DRILLED PRODUCT (Soil type, colour, grain size, plasticity, minor components, observations)	S Y M B O L	CONSISTENCY (cohesive soils) or RELATIVE DENSITY (sands and gravels)	M O I S T U R E
	S3/1 @ 0.2 m	-		SILTY CLAY: brown with light brown, low to medium plasticity, trace of gravel TOPSOIL	CL	FIRM TO STIFF	D-M
	S3/2 @ 0.8 m	1.0		SILTY CLAY: light brown with light grey and some light orange, medium to high plasticity, trace of g	rave CL/C	H STIFF	М
	S3/3 @ 1.6 m	2.0		SILTY CLAY: grey with light grey and some light brown, low to medium plasticity, trace of gravel	CL	VERY STIFF	D-M
		4.0		WEATHERED SHALE; grey with light grey AUGER REFUSAL AT 3.2 M ON WEATHERED SHALE		EXTREMELY LOW STRENGTH	
NOTES:	D - disturbe WT - level o S - jar samp	of water		U - undisturbed tube sample B - bulk sample free water N - Standard Penetration Test (SPT) See explanation sheets for meaning of all descriptive terms and symbols	Hole Dia	or: STS nt: Christie nmeter (mm): 100/200/300 m Vertical (°): :: V/Spiral/Two Prong	0

Project:	MJ Demolitio 55 Martin Ro Refer to Drav	oad, Bad	gerys C	reek Date: December 12, 2017	ВС	Sheet 1 of 1	BH 4
W A T T A E B R L E	S A M P L E S	DEF	тн	DESCRIPTION OF DRILLED PRODUCT (Soil type, colour, grain size, plasticity, minor components, observations)	S Y M B O L	CONSISTENCY (cohesive soils) or RELATIVE DENSITY (sands and gravels)	M O I S T U R E
	S4/1 @ 0.2 m			SILTY CLAY: brown with dark brown, low to medium plasticity, trace of gravel	CL	STIFF	D-M
	S4/2 @ 0.7 m S4/3 @ 0.9 m			TOPSOIL SILTY CLAY: light brown with orange brown, low to medium plasticity, trace of gravel, trace of fine grained sand	CL	STIFF	M
	B 1.0-1.4 m	1.0		SILTY CLAY: orange brown with light grey and some light brown, medium to high plasticity, trace of gravel	CL/CH	VERY STIFF	М
	S4/4 @ 1.4 m			SILTY CLAY: light grey with light brown, medium to high plasticity, trace of gravel	CL/CH	VERY STIFF	M
	S4/5 @ 2.1 m	2.0		SILTY CLAY: light brown with grey and some light grey, low to medium plasticity, trace of shale	CL	VERY STIFF	M
	S4/6 @ 3.0 m	3.0		SILTY CLAY: grey with light grey, low to medium plasticity, trace of shale	CL	VERY STIFF	M
				WEATHERED SHALE: grey with dark grey AUGER REFUSAL AT 3.8 M ON WEATHERED SHALE		EXTREMELY LOW STRENGTH	
		4.0		AUGER REPUSAL AT 5.8 M ON WEATHERED SHALE			
		5.0					
	D - disturbe WT - level of S - jar samp	of water		U - undisturbed tube sample B - bulk sample free water N - Standard Penetration Test (SPT)		:: STS t: Christie neter (mm): 100/200/300)
NOTES:	Jap			See explanation sheets for meaning of all descriptive terms and symbols	Angle from	a Vertical (°): V/Spiral/Two Prong	

Project:	55 Martin Ro	n and Excavationad, Badgerys C	reek Date: December 12, 2017	ВС	Sheet 1 of 1	BH 5
W A T T A E B R L E	S A M P L E S	DEPTH (m)	DESCRIPTION OF DRILLED PRODUCT (Soil type, colour, grain size, plasticity, minor components, observations)	S Y M B O L	CONSISTENCY (cohesive soils) or RELATIVE DENSITY (sands and gravels)	M O I S T U R E
	S5/1 @ 0.2 m		SILTY CLAY: brown with dark brown, low to medium plasticity, trace of gravel TOPSOIL	CL		D
		1.0	BOREHOLE DISCONTINUED AT 0.3 M			
	D - disturbe WT - level o	d sample of water table or	$U \text{ - undisturbed tube sample} \qquad B \text{ - bulk sample} \\ \text{free water} \qquad \qquad N \text{ - Standard Penetration Test (SPT)}$	Contractor Equipmen	r: STS t: Christie	
NOTES:	S - jar samp	le	See explanation sheets for meaning of all descriptive terms and symbols	Angle fron	neter (mm): 100/200/300 n Vertical (°): V/Spiral/Two Prong)

Client: AM	/J Demolition	n and Excavation	n P/L Project / STS No.: 21649/8653C	REHOLE NO.:	BH 6	
_		ad, Badgerys C				
Location: R	Refer to Draw	ing No. 17/390	5 Logged: DL Checked By: MG		Sheet 1 of 1	1
W A T T A E B R L E	S A M P L E	DEPTH	DESCRIPTION OF DRILLED PRODUCT (Soil type, colour, grain size, plasticity, minor components, observations)	S Y M B O L	CONSISTENCY (cohesive soils) or RELATIVE DENSITY (sands and gravels)	M O I S T U R
	S6/1	(m)	CH TV CI AV, heavy with deal heavy low to modium ploticity tross of served		CTIEF	
	\$6/1 @ 0.2 m		SILTY CLAY: brown with dark brown, low to medium plasticity, trace of gravel TOPSOIL	CL	STIFF	D
	@ 0.6 m		SILTY CLAY: light brown with light grey, low to medium plasticity, trace of gravel	CL	VERY STIFF	D-M
	U50	1.0				
	S6/3		CWTW CV AV PL	CV (CV)	AVEDAL CONTE	
	@ 1.6 m	2.0	SILTY CLAY: light grey with grey, medium to high plasticity, trace of gravel	CL/CH	VERY STIFF	М
	S6/4 @ 2.4 m		SILTY CLAY: light brown with light grey, low to medium plasticity, trace of gravel	CL	VERY STIFF	D-M
		3.0				
			WEATHERED SHALE: light brown with brown		EXTREMELY LOW	
		4.0	AUGER REFUSAL AT 3.3 M ON WEATHERED SHALE		STRENGTH	
		5.0				
	D - disturbed		•	Contractor		
		of water table or		Equipment		0
NOTES:	S - jar sampl	le		Angle from	eter (mm): 100/200/30 Vertical (°): V/Spiral/Two Prong	U

		n and Excavationad, Badgerys C	-	ВО	REHOLE NO.:	BH 7
		ving No. 17/390			Sheet 1 of 1	
W A T T A E B R L E	S A M P L E	DEPTH (m)	DESCRIPTION OF DRILLED PRODUCT (Soil type, colour, grain size, plasticity, minor components, observations)	S Y M B O L	consistency (cohesive soils) or RELATIVE DENSITY (sands and gravels)	M O I S T U R
	S7/1 @ 0.2 m		SILTY CLAY: brown with light brown, low to medium plasticity, trace of gravel	CL	STIFF	D-M
	S7/2 @ 0.7 m		TOPSOIL SILTY CLAY: light brown with light grey, low to medium plasticity, trace of gravel	CL	VERY STIFF	D-M
		1.0				
	S7/3 @ 1.6 m	2.0	SILTY CLAY: light grey with light brown, medium to high plasticity, trace of gravel	CL/CH	VERY STIFF	M
	S7/4 @ 2.8 m	3.0	SILTY CLAY: grey with light grey and some orange brown, low to medium plasticity, trace of shale	CL	VERY STIFF	M
			WEATHERED SHALE: grey with dark grey AUGER REFUSAL AT 3.6 M ON WEATHERED SHALE		EXTREMELY LOW STRENGTH	
		5.0				
	D - disturbe WT - level o S - jar samp	of water table or	free water N - Standard Penetration Test (SPT)		: STS :: Christie seter (mm): 100/200/300	0
NOTES:					Vertical (°): V/Spiral/Two Prong	

		n and Excavation		ВО	REHOLE NO.:	BH 8
		oad, Badgerys C ving No. 17/390			Sheet 1 of 1	
Location: I	Refer to Drav	ving No. 17/390	5 Logged: JK Checked By: MG		CONSISTENCY	М
W A T T A E B R L	S A M P L		DESCRIPTION OF DRILLED PRODUCT	S Y M B	or RELATIVE DENSITY (sands and	O I S T U
E	E S	DEPTH (m)	(Soil type, colour, grain size, plasticity, minor components, observations)	O L	gravels)	R E
	\$1/DUP/TRI @ 0.2 m		SILTY CLAY: dark brown, low plasticity	CL	FIRM TO STIFF	D
	S8/2 @ 0.5 m		TOPSOIL SILTY CLAY: orange brown with light grey, medium to high plasticity	CL/CH	STIFF	M
	B @0.3-0.9m \$8/3					
	@ 1.0m	1.0	SILTY CLAY: light grey with yellow brown/orange brown, medium to high plasticity	CL/CH	STIFF	М
	S8/4 @ 1.5 m					
	S8/5				VERY STIFF	-
	@ 2.0 m	2.0				
	\$8/6 @ 2.5 m					
WT 18/12/17	-		WEATHERED SHALE: dark grey with occasional light grey, trace of fine grained sand		EXTREMELY LOW STRENGTH	D
	S8/7 @ 3.0 m	3.0				
	50.10					
	S8/8 @ 4.0 m	4.0				
		5.0				
			CTANIDDIDE DIEZOMETED INCTALLED			
			STANDPIPE PIEZOMETER INSTALLED			
	D - disturbe		BOREHOLE DISCONTINUED AT 6.0 M U - undisturbed tube sample B - bulk sample	Contractor	: STS	
	WT - level o	of water table or		Equipment	: Edson RP70	
	S - jar samp	le	See explanation sheets for meaning of all descriptive terms and symbols	-	eter (mm): 100 Vertical (°):	
NOTES:			oce explanation success for incuming of an descriptive terms and symbols	Angle from Drill Bit:		
<u> </u>				I		

		n and Excavation		ВО	REHOLE NO.:	ВН 9
		ving No. 17/390			Sheet 1 of 1	
W A T T A E B R L E	S A M P L E S	DEPTH (m)	DESCRIPTION OF DRILLED PRODUCT (Soil type, colour, grain size, plasticity, minor components, observations)	S Y M B O L	CONSISTENCY (cohesive soils) or RELATIVE DENSITY (sands and gravels)	M O I S T U R E
	S9/1 @ 0.2 m		SILTY CLAY: dark brown, low plasticity	CL	FIRM TO STIFF	D
			TOPSOIL SILTY CLAY: orange brown with light grey, medium to high plasticity	CL/CH	STIFF	M-D
		1.0	SILT F CLAT: Gainge from with right grey, medium to high plasticity	CLICH	51111	WI-D
			SILTY CLAY: light grey with orange brown, medium to high plasticity	CL/CH	VERY STIFF	M
		2.0	WEATHERED SHALE: light grey with dark grey, fine grained, clay seams		EXTREMELY LOW STRENGTH	D
		3.0				
		5.0	AUGER REFUSAL AT 4.0 M ON WEATHERED SHALE			
		of water table or	free water N - Standard Penetration Test (SPT)		: Edson RP70	
NOTES:	S - jar sampl	le		Angle from	eter (mm): 100/200/300 Vertical (°): V/Spiral/Two Prong)

Discribing Dis	Client: AN	MJ Demolitio	n and Excavation	n P/L Project / STS No.: 21649/8653C	ВО	REHOLE NO.:	BH 10
Consistency Management Consistency Consiste	II					Chart 1 of 1	
Note: Note: Note:	Location: I	keier to Drav	ving No. 17/390	Logged: DL Checked By: MG	+		
D - disturbed stample U - undisturbed tube sample B - bulk sample NOTISS See explanation sheets for meaning of all descriptive terms and symbols TOISOIL TOISOIL Logical Tools and the sample and	A T T A E B R L	A M P L			Y M B O	or RELATIVE DENSITY (sands and	O I S T U R
BORBHOLE DISCONTINUED AT 0.3 M 1.0 2.0 3.0 3.0 4.0 5.0 1.0 Ddisumbed sample Wit - keet of water table or free water Wit - keet of water table or free water S. y as sample NOTISS: See explanation sheets for meaning of all descriptive terms and symbols Angle from Vertical (): Contractor: STN Explanation Test (SPT) Angle from Vertical ():				SILTY CLAY: brown with dark brown, low to medium plasticity, trace of gravel	CL		D
D - disturbed sample WIT - level of water table or free water WT - level of water table or free water S - jar sample NOTES: See explanation sheets for meaning of all descriptive terms and symbols Augle from Vertical Cr. Contractor: STS Expigneers: Christie Hold Disturber (mile) (10/200/300 Augle from Vertical Cr.		@ 0.2 m					
D - disturbed sample WT - level of water table or free water S - jur sample NOTES: See explanation sheets for meaning of all descriptive terms and symbols Augle from Vertical (7): Augle from Vertical (7):				BOREHOLE DISCONTINUED AT 0.3 M			
D - disturbed sample WT - level of water table or free water S - jur sample NOTES: See explanation sheets for meaning of all descriptive terms and symbols Augle from Vertical (7): Augle from Vertical (7):							
D - disturbed sample WT - level of water table or free water S - jur sample NOTES: See explanation sheets for meaning of all descriptive terms and symbols Augle from Vertical (7): Augle from Vertical (7):							
D - disturbed sample WT - level of water table or free water S - jur sample NOTES: See explanation sheets for meaning of all descriptive terms and symbols Augle from Vertical (7): Augle from Vertical (7):			1.0				
D - disturbed sample							
D - disturbed sample							
D - disturbed sample			_				
D - disturbed sample							
D - disturbed sample			20				
D - disturbed sample							
D - disturbed sample U - undisturbed tube sample B - bulk sample WT - level of water table or free water N - Standard Penetration Test (SPT) See explanation sheets for meaning of all descriptive terms and symbols See explanation sheets for meaning of all descriptive terms and symbols Angle from Vertical (*):							
D - disturbed sample U - undisturbed tube sample B - bulk sample WT - level of water table or free water N - Standard Penetration Test (SPT) See explanation sheets for meaning of all descriptive terms and symbols See explanation sheets for meaning of all descriptive terms and symbols Angle from Vertical (*):							
D - disturbed sample U - undisturbed tube sample B - bulk sample WT - level of water table or free water N - Standard Penetration Test (SPT) See explanation sheets for meaning of all descriptive terms and symbols See explanation sheets for meaning of all descriptive terms and symbols Angle from Vertical (*):							
D - disturbed sample							
D - disturbed sample WT - level of water table or free water N - Standard Penetration Test (SPT) S - jar sample NOTES: See explanation sheets for meaning of all descriptive terms and symbols Region of the descriptive terms and symbols Region of the descriptive terms and symbols Region of the descriptive terms and symbols Angle from Vertical (°):			3.0				
D - disturbed sample WT - level of water table or free water N - Standard Penetration Test (SPT) S - jar sample NOTES: See explanation sheets for meaning of all descriptive terms and symbols Region of the descriptive terms and symbols Region of the descriptive terms and symbols Region of the descriptive terms and symbols Angle from Vertical (°):							
D - disturbed sample WT - level of water table or free water N - Standard Penetration Test (SPT) S - jar sample NOTES: See explanation sheets for meaning of all descriptive terms and symbols Region of the descriptive terms and symbols Region of the descriptive terms and symbols Region of the descriptive terms and symbols Angle from Vertical (°):							
D - disturbed sample WT - level of water table or free water N - Standard Penetration Test (SPT) S - jar sample NOTES: See explanation sheets for meaning of all descriptive terms and symbols Region of the descriptive terms and symbols Region of the descriptive terms and symbols Region of the descriptive terms and symbols Angle from Vertical (°):							
D - disturbed sample WT - level of water table or free water N - Standard Penetration Test (SPT) S - jar sample NOTES: See explanation sheets for meaning of all descriptive terms and symbols Region of the descriptive terms and symbols Region of the descriptive terms and symbols Region of the descriptive terms and symbols Angle from Vertical (°):							
D - disturbed sample WT - level of water table or free water S - jar sample NOTES: See explanation sheets for meaning of all descriptive terms and symbols U - undisturbed tube sample B - bulk sample Contractor: STS Equipment: Christie Hole Diameter (mm): 100/200/300 Angle from Vertical (°):			4.0				
D - disturbed sample WT - level of water table or free water S - jar sample NOTES: See explanation sheets for meaning of all descriptive terms and symbols U - undisturbed tube sample B - bulk sample Contractor: STS Equipment: Christie Hole Diameter (mm): 100/200/300 Angle from Vertical (°):			_				
D - disturbed sample WT - level of water table or free water S - jar sample NOTES: See explanation sheets for meaning of all descriptive terms and symbols U - undisturbed tube sample B - bulk sample Contractor: STS Equipment: Christie Hole Diameter (mm): 100/200/300 Angle from Vertical (°):							
D - disturbed sample WT - level of water table or free water S - jar sample NOTES: See explanation sheets for meaning of all descriptive terms and symbols U - undisturbed tube sample B - bulk sample Contractor: STS Equipment: Christie Hole Diameter (mm): 100/200/300 Angle from Vertical (°):							
D - disturbed sample WT - level of water table or free water S - jar sample NOTES: See explanation sheets for meaning of all descriptive terms and symbols U - undisturbed tube sample B - bulk sample Contractor: STS Equipment: Christie Hole Diameter (mm): 100/200/300 Angle from Vertical (°):							
D - disturbed sample WT - level of water table or free water S - jar sample NOTES: See explanation sheets for meaning of all descriptive terms and symbols U - undisturbed tube sample B - bulk sample Contractor: STS Equipment: Christie Hole Diameter (mm): 100/200/300 Angle from Vertical (°):							
WT - level of water table or free water S - jar sample N - Standard Penetration Test (SPT) Equipment: Christie Hole Diameter (mm): 100/200/300 Angle from Vertical (°):			5.0				
WT - level of water table or free water S - jar sample N - Standard Penetration Test (SPT) Equipment: Christie Hole Diameter (mm): 100/200/300 Angle from Vertical (°):							
WT - level of water table or free water S - jar sample N - Standard Penetration Test (SPT) Equipment: Christie Hole Diameter (mm): 100/200/300 Angle from Vertical (°):							
WT - level of water table or free water S - jar sample N - Standard Penetration Test (SPT) Equipment: Christie Hole Diameter (mm): 100/200/300 Angle from Vertical (°):							
WT - level of water table or free water S - jar sample N - Standard Penetration Test (SPT) Equipment: Christie Hole Diameter (mm): 100/200/300 Angle from Vertical (°):							
WT - level of water table or free water S - jar sample N - Standard Penetration Test (SPT) Equipment: Christie Hole Diameter (mm): 100/200/300 Angle from Vertical (°):							
S - jar sample NOTES: See explanation sheets for meaning of all descriptive terms and symbols Hole Diameter (mm): 100/200/300 Angle from Vertical (°):				•			
NOTES: See explanation sheets for meaning of all descriptive terms and symbols Angle from Vertical (°):							0
	NOTES:						
<u>. </u>					Drill Bit:	V/Spiral/Two Prong	

Project:	55 Martin Ro	n and Excavationad, Badgerys C	reek Date: December 12, 2017	ВО	Sheet 1 of 1	BH 11
W A T T A E B R L E	S A M P L E S	DEPTH (m)	DESCRIPTION OF DRILLED PRODUCT (Soil type, colour, grain size, plasticity, minor components, observations)	S Y M B O L	CONSISTENCY (cohesive soils) or RELATIVE DENSITY (sands and gravels)	M O I S T U R E
	S1/DUP/TRI @ 0.2 m		SILTY CLAY: dark brown/orange brown, medium plasticity	CL	FIRM TO STIFF	D-M
		1.0	SILTY CLAY: orange brown with light grey, medium to high plasticity	CL/CH	STIFF	M
		2.0	SILTY CLAY: light grey with orange brown and yellow brown, medium plasticity, trace of fine grained sand	CL	VERY STIFF	M-D
			WEATHERED SHALE: light brown with orange brown and dark grey, fine grained, clay seams		EXTREMELY LOW STRENGTH	D
		5.0	AUGER REFUSAL AT 4.5 M ON WEATHERED SHALE			
	D - disturbed WT - level of S - jar samp	of water table or	U - undisturbed tube sample Free water B - bulk sample N - Standard Penetration Test (SPT)		:: STS t: Edson RP70 neter (mm): 100	
NOTES:			See explanation sheets for meaning of all descriptive terms and symbols	Angle from	n Vertical (°): Spiral	

Client: AN	MJ Demolitio	n and Excavation	n P/L Project / STS No.: 21649/8653C	ВО	REHOLE NO.:	BH 12
II		ad, Badgerys C			Chest 1 of 1	
Location: I	keier to Drav	ving No. 17/390	5 Logged: DL Checked By: MG	+	Sheet 1 of 1	
W A T T A E B R L E	S A M P L E	DEPTH (m)	DESCRIPTION OF DRILLED PRODUCT (Soil type, colour, grain size, plasticity, minor components, observations)	S Y M B O L	consistency (cohesive soils) or RELATIVE DENSITY (sands and gravels)	M O I S T U R E
	S12/1	_	SILTY CLAY: brown with dark brown, low to medium plasticity, trace of gravel	CL		D
	@ 0.2 m		TOPSOIL			
		1.0	SOREHOLE DISCONTINUED AT 0.3 M			
	D - disturbe	d sample	U - undisturbed tube sample B - bulk sample	Contractor	: STS	
		of water table or			: Christie	
NOTES:	S - jar samp	le	See explanation sheets for meaning of all descriptive terms and symbols	angle from	eter (mm): 100/200/30 Vertical (°):	0
				חום חוני	V/Spiral/Two Prong	

Project:	55 Martin Ro	n and Excavationad, Badgerys C	eek Date: December 12, 2017	ВС	OREHOLE NO.:	BH 13
W A T T A E B R L E	S A M P L E S	ving No. 17/390 DEPTH (m)	Logged: DL Checked By: MG DESCRIPTION OF DRILLED PRODUCT (Soil type, colour, grain size, plasticity, minor components, observations)	S Y M B O L	Sheet 1 of 1 CONSISTENCY (cohesive soils) or RELATIVE DENSITY (sands and gravels)	M O I S T U R E
	S S12/1 @ 0.2 m B1 @ 0.4 m	1.0	TOPSOIL SILTY CLAY: light brown with orange brown, low to medium plasticity, trace of gravel SOREHOLE DISCONTINUED AT 1.5 M	CL CL		E D D
	D - disturbe WT - level o S - jar samp	of water table or			r: STS t: Christie neter (mm): 100/200/30	00
NOTES:			See explanation sheets for meaning of all descriptive terms and symbols		n Vertical (°): V/Spiral/Two Prong	

Project:	55 Martin Ro	n and Excavationad, Badgerys C	eek Date: December 12, 2017			BH 14
W A T T A E B R L E	S A M P L E S	ving No. 17/390 DEPTH (m)	DESCRIPTION OF DRILLED PRODUCT (Soil type, colour, grain size, plasticity, minor components, observations)	S Y M B O L	Sheet 1 of 1 CONSISTENCY (cohesive soils) or RELATIVE DENSITY (sands and gravels)	M O I S T U R E
	S14/1 @ 0.2 m B2 @ 0.4 m S14/2 @ 0.9 m	1.0	TOPSOIL SILTY CLAY: light brown with orange brown, low to medium plasticity, trace of gravel SILTY CLAY: light brown with orange brown, low to medium plasticity, trace of gravel BOREHOLE DISCONTINUED AT 1.5 M	CL		D D
	D - disturbe WT - level o S - jar samp	of water table or	U - undisturbed tube sample B - bulk sample free water N - Standard Penetration Test (SPT)	Contractor Equipment Hole Diam		0
NOTES:			See explanation sheets for meaning of all descriptive terms and symbols		Vertical (°): V/Spiral/Two Prong	

Project:	55 Martin Ro	n and Excavationad, Badgerys C	reek Date: December 12, 2017	ВО	REHOLE NO.:	BH 15
W A T T A E B R L E	S A M P L E S	ving No. 17/390 DEPTH (m)	DESCRIPTION OF DRILLED PRODUCT (Soil type, colour, grain size, plasticity, minor components, observations)	S Y M B O L	Sheet 1 of 1 CONSISTENCY (cohesive soils) or RELATIVE DENSITY (sands and gravels)	M O I S T U R E
	B4/S15-1 @ 0.2 m		SILTY CLAY: dark brown, low plasticity TOPSOIL	CL	FIRM	D
	S15/2 @ 0.5 m U50 S15/3 @ 1.0 m	1.0	SILTY CLAY: orange brown with light grey, medium to high plasticity	CL/CH	STIFF VERY STIFF	D-M
	S15/4 @ 1.5 m		WEATHERED SANDSTONE: dark grey with light grey and orange brown, fine grained, clay seams			D
	@ 2.0 m S15/6 @ 2.5 m	2.0				
	S15/7 @ 3.0 m	3.0				D-M
	S15/8 @ 4.0 m	4.0	AUGER REFUSAL AT 4.3 M ON WEATHERD SANDSTONE			D
		5.0	STANDPIPE PIEZOMETER INSTALLED			
	D - disturbed WT - level of S - jar samp	of water table or	U - undisturbed tube sample B - bulk sample free water N - Standard Penetration Test (SPT)		: STS : Edson RP70 eter (mm): 100	
NOTES:			See explanation sheets for meaning of all descriptive terms and symbols	Angle from Drill Bit: S	Vertical (°): Spiral	

	molition and Excavation		ВО	REHOLE NO.:	BH 16
	artin Road, Badgerys C to Drawing No. 17/390			Sheet 1 of 1	
W S A T A M E B B B B B B B B B B B B B B B B B B	S A M P L L E DEPTH S (m)	DESCRIPTION OF DRILLED PRODUCT (Soil type, colour, grain size, plasticity, minor components, observations)	S Y M B O L	CONSISTENCY (cohesive soils) or RELATIVE DENSITY (sands and gravels)	M O I S T U R
S1	0.2 m	SILTY CLAY: dark brown, low plasticity TOPSOIL SOREHOLE DISCONTINUED AT 0.2 M			
WT -	listurbed sample - level of water table or ur sample	U - undisturbed tube sample B - bulk sample N - Standard Penetration Test (SPT)		:: STS t: Edson RP70 neter (mm): 100	
NOTES:		See explanation sheets for meaning of all descriptive terms and symbols		a Vertical (°):	

Project: 5	55 Martin Ro	n and Excavationad, Badgerys C	peek Date: December 12, 2017	ВС	PREHOLE NO.:	BH 17
W A T T A E B R L E	S A M P L E S	DEPTH (m)	DESCRIPTION OF DRILLED PRODUCT (Soil type, colour, grain size, plasticity, minor components, observations)	S Y M B O L	Sheet 1 of 1 CONSISTENCY (cohesive soils) or RELATIVE DENSITY (sands and gravels)	M O I S T U R E
	S17/1 @ 0.2 m B		SILTY CLAY: dark brown, low plasticity	CL	FIRM TO STIFF	D
	0.4-1.0		TOPSOIL SILTY CLAY: orange brown with light grey, medium to high plasticity	CL/CH	STIFF	M
		1.0	SANDY CLAY: light grey with orange brown, fine grained sand, medium plasticity	CL	STIFF VERY STIFF	M-D
		2.0				M
		3.0				
			WEATHERED SHALE: light grey with orange brown and yellow brown, trace of fined grained sand		EXTREMELY LOW STRENGTH	D
		4.0				
		5.0	AUGER REFUSAL AT 5.0 M ON WEATHERED SHALE			
	D - disturbed WT - level of S - jar sampl	of water table or	U - undisturbed tube sample B - bulk sample free water N - Standard Penetration Test (SPT)		r: STS t: Edson RP70 neter (mm): 100	
NOTES:			See explanation sheets for meaning of all descriptive terms and symbols	Angle fron	n Vertical (°): Spiral	

Client: AMJ Demoliti Project: 55 Martin R Location: Refer to Dra	toad, Badgerys C	peek Date: December 12, 2017	ВО	Sheet 1 of 1	BH 18
W S A T A M E B P R L L E S S	DEPTH (m)	DESCRIPTION OF DRILLED PRODUCT (Soil type, colour, grain size, plasticity, minor components, observations)	S Y M B O L	CONSISTENCY (cohesive soils) or RELATIVE DENSITY (sands and gravels)	M O I S T U R E
S18/1 @ 0.2 m		TOPSOIL SOREHOLE DISCONTINUED AT 0.3 M			
D - disturb WT - level S - jar sam NOTES:	of water table or	U - undisturbed tube sample B - bulk sample free water N - Standard Penetration Test (SPT) See explanation sheets for meaning of all descriptive terms and symbols	Hole Diam	:: STS :: Christie neter (mm): 100/200/30 i Vertical (°): V/Spiral/Two Prong	0

Client: AMJ Demolition and Excavation P/L			n P/L Project / STS No.: 21649/8653C	ВО	BH 19	
II		ad, Badgerys C			Chart 1 of 1	
Location: I	keier to Drav	ving No. 17/390	5 Logged: DL Checked By: MG		Sheet 1 of 1	
W A T T A E B R L E	S A M P L E	DEPTH (m)	DESCRIPTION OF DRILLED PRODUCT (Soil type, colour, grain size, plasticity, minor components, observations)	S Y M B O L	consistency (cohesive soils) or RELATIVE DENSITY (sands and gravels)	M O I S T U R E
	S19/1		SILTY CLAY: brown with dark brown, low to medium plasticity, trace of gravel	CL		D
	@ 0.2 m		TOPSOIL			
			BOREHOLE DISCONTINUED AT 0.3M			
		1.0				
		_				
		2.0				
		<u> </u>				
		3.0				
		4.0				
		5.0				
	D - disturbe	d sample of water table or	•	Contractor	: STS : Christie	
	S - jar samp				eter (mm): 100/200/30	0
NOTES:	3 ··· T				Vertical (°):	
1,0113.					V/Spiral/Two Prong	
1						

14/1 Cowpasture Place, Wetherill Park NSW 2164

Phone: (02)9756 2166 Fax: (02)9756 1137 Email: enquiries@smectesting.com.au



Dynamic Cone Penetrometer Test Report

Project: No.55 Martin Road, Badgerys Creek
Client: AMJ Demolition and Excavation P/L

Address: No.44 Pearson Street, South Wentworthville 2145

Test Method: AS 1289.6.3.2

Project No.: 21649/8653C Report No.: 17/3905 Report Date: 15/12/2017

Page: 1 of 3

Site No.	P1	P2	P3	P4		P1	P2	P3	P4
Location	Refer to Drawing No. 17/3905	Refer to Drawing No. 17/3905	Refer to Drawing No. 17/3905	Refer to Drawing No. 17/3905					
Starting Level	Surface Level	Surface Level	Surface Level	Surface Level					
Depth (m)	Penetr	ration Resista	nce (blows / 1	50mm)	Depth (m)	Penet	ration Resista	ance (blows / 1	50mm)
0.00 - 0.15	3	3	5	5	3.00 - 3.15			*	
0.15 - 0.30	5	5	8	10	3.15 - 3.30			*	
0.30 - 0.45	6	7	9	14	3.30 - 3.45			*	
0.45 - 0.60	6	6	10	16	3.45 - 3.60			*	
0.60 - 0.75	7	6	11	13	3.60 - 3.75			22	
0.75 - 0.90	5	9	12	11	3.75 - 3.90			Refusal	
0.90 - 1.05	5	16	12	15	3.90 - 4.05				
1.05 - 1.20	6	16	13	14	4.05 - 4.20				
1.20 - 1.35	8	20	22	22	4.20 - 4.35				
1.35 - 1.50	11	22	*	*	4.35 - 4.50				
1.50 - 1.65	11	*	*	*	4.50 - 4.65				
1.65 - 1.80	15	*	*	*	4.65 - 4.80				
1.80 - 1.95	19	*	*	*	4.80 - 4.95				
1.95 - 2.10	22	*	22	*	4.95 - 5.10				
2.10 - 2.25	Refusal	*	*	22	5.10 - 5.25				
2.25 - 2.40		22	*	Refusal	5.25 - 5.40				
2.40 - 2.55		Refusal	*		5.40 - 5.55				
2.55 - 2.70			*		5.55 - 5.70				
2.70 - 2.85			*		5.70 - 5.85				
2.85 - 3.00			22		5.85 - 6.00				

Remarks:

Technician: DL/JK

^{* =} Pre-drilled hole prior to testing



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dards Approve

Approved Signatory......Laurie Ihnativ - Manager

Form: RPS26Long

Date of Issue: 01/06/15 Revision: 5

14/1 Cowpasture Place, Wetherill Park NSW 2164

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Dynamic Cone Penetrometer Test Report

Project: No.55 Martin Road, Badgerys Creek

Client: AMJ Demolition and Excavation P/L

Address: No.44 Pearson Street, South Wentworthville 2145

Test Method: AS 1289.6.3.2

Project No.: 21649/8653C Report No.: 17/3905

Report Date: 15/12/2017 Page: 2 of 3

Site No.	P5	P6	P7	P8		P5	P6	P7	P8
Location	Refer to Drawing No. 17/3905	Refer to Drawing No. 17/3905	Refer to Drawing No. 17/3905	Refer to Drawing No. 17/3905					
Starting Level	Surface Level	Surface Level	Surface Level	Surface Level					
Depth (m)	Penetr	ation Resista	nce (blows / 1	50mm)	Depth (m)	Pene	tration Resista	ance (blows / '	150mm)
0.00 - 0.15	6	2	2	2	3.00 - 3.15				
0.15 - 0.30	9	3	4	3	3.15 - 3.30				
0.30 - 0.45	13	3	4	5	3.30 - 3.45				
0.45 - 0.60	13	5	5	5	3.45 - 3.60				
0.60 - 0.75	13	7	6	6	3.60 - 3.75				
0.75 - 0.90	16	8	6	7	3.75 - 3.90				
0.90 - 1.05	15	8	10	5	3.90 - 4.05				
1.05 - 1.20	14	9	8	8	4.05 - 4.20				
1.20 - 1.35	12	11	17	12	4.20 - 4.35				
1.35 - 1.50	18	13	22	9	4.35 - 4.50				
1.50 - 1.65	16	9	Refusal	10	4.50 - 4.65				
1.65 - 1.80	22	9		13	4.65 - 4.80				
1.80 - 1.95	*	12		13	4.80 - 4.95				
1.95 - 2.10	*	22		10	4.95 - 5.10				
2.10 - 2.25	*	Refusal		17	5.10 - 5.25				
2.25 - 2.40	*			22	5.25 - 5.40				
2.40 - 2.55	18			Refusal	5.40 - 5.55				
2.55 - 2.70	19				5.55 - 5.70				
2.70 - 2.85	22				5.70 - 5.85				
2.85 - 3.00	Refusal				5.85 - 6.00				

Technician: DL/JK

* = Pre-drilled hole prior to testing

Remarks:

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Laurie Ihnativ - Manager

Form: RPS26Long Date of Issue: 01/06/15 Revision: 5

14/1 Cowpasture Place, Wetherill Park NSW 2164

Phone: (02)9756 2166 Fax: (02)9756 1137 Email: enquiries@smectesting.com.au



Dynamic Cone Penetrometer Test Report

Project: No.55 Martin Road, Badgerys Creek Client: AMJ Demolition and Excavation P/L

Address: No.44 Pearson Street, South Wentworthville 2145

Test Method: AS 1289.6.3.2

Report No.: 17/3905 Report Date: 15/12/2017 Page: 3 of 3

Project No.: 21649/8653C

						,			
Site No.	P9	P10							
Location	Refer to Drawing No. 17/3905	Refer to Drawing No. 17/3905							
Starting Level	Surface Level	Surface Level							
Depth (m)	Penetr	ation Resistar	nce (blows / 1	50mm)	Depth (m)	Penet	ration Resista	nce (blows / 1	50mm)
0.00 - 0.15	2	3			3.00 - 3.15				
0.15 - 0.30	1	4			3.15 - 3.30				
0.30 - 0.45	3	5			3.30 - 3.45				
0.45 - 0.60	4	5			3.45 - 3.60				
0.60 - 0.75	6	8			3.60 - 3.75				
0.75 - 0.90	10	10			3.75 - 3.90				
0.90 - 1.05	12	9			3.90 - 4.05				
1.05 - 1.20	15	9			4.05 - 4.20				
1.20 - 1.35	22	12			4.20 - 4.35				
1.35 - 1.50	Refusal	19			4.35 - 4.50				
1.50 - 1.65		22			4.50 - 4.65				
1.65 - 1.80		Refusal			4.65 - 4.80				
1.80 - 1.95					4.80 - 4.95				
1.95 - 2.10					4.95 - 5.10				
2.10 - 2.25		_			5.10 - 5.25				
2.25 - 2.40					5.25 - 5.40				
2.40 - 2.55				_	5.40 - 5.55				
2.55 - 2.70					5.55 - 5.70				
2.70 - 2.85					5.70 - 5.85				
2.85 - 3.00					5.85 - 6.00				

Remarks:

^{* =} Pre-drilled hole prior to testing



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Approved Signatory..... Laurie Ihnativ - Manager

Technician: DL/JK

Form: RPS26Long Date of Issue: 01/06/15 Revision: 5

E1. CLASSIFICATION OF SOILS

E1.1 Soil Classification and the Unified System

An assessment of the site conditions usually includes an appraisal of the data available by combining values of engineering properties obtained by the site investigation with descriptions, from visual observation of the materials present on site.

The system used by SMEC in the identification of soil is the Unified Soil Classification system (USC) which was developed by the US Army Corps of Engineers during World War II and has since gained international acceptance and has been adopted in its metricated form by the Standards Association of Australia.

The Australian Site Investigation Code (AS1726-1981, Appendix D) recommends that the description of a soil includes the USC group symbols which are an integral component of the system.

The soil description should contain the following information in order:

Soil composition

- SOIL NAME and USC classification symbol (IN BLOCK LETTERS)
- plasticity or particle characteristics
- colour
- secondary and minor constituents (name estimated proportion, plasticity or particle characteristics, colour

Soil condition

- moisture condition
- consistency or density index

Soil structure

• structure (zoning, defects, cementing)

Soil origin

interpretation based on observation eg FILL, TOPSOIL, RESIDUAL, ALLUVIUM.

E1.2 Soil Composition

(a) Soil Name and Classification Symbol

The USC system is summarised in Figure E1.2.1. The primary division separates soil types on the basis of particle size into:

- Coarse grained soils more than 50% of the material less than 60 mm is larger than 0.06 mm (60 μm).
- Fine grained soils more than 50% of the material less than 60 mm is smaller than 0.06 mm (60 μ m).

Initial classification is by particle size as shown in Table E1.2.1. Further classification of fine grained soils is based on plasticity.

TABLE E1.2.1 - CLASSIFICATION BY PARTICLE SIZE

NAME	SUB-DIVISION	SIZE
Clay (1)		< 2 μm
Silt (2)		2 μm to 60 μm
Sand	Fine Medium Coarse	60 μm to 200 μm 200 μm to 600 μm 600 μm to 2 mm
Gravel (3)	Fine Medium Coarse	2 mm to 6 mm 6 mm to 20 mm 20 mm to 60 mm
Cobbles (3)		60 mm to 200 mm
Boulders (3)		> 200 mm

Where a soil contains an appropriate amount of secondary material, the name includes each of the secondary components (greater than 12%) in increasing order of significance, eg sandy silty clay.

Minor components of a soil are included in the description by means of the terms "some" and "trace" as defined in Table E1.2.2.

TABLE E1.2.2 - MINOR SOIL COMPONENTS

TERM	DESCRIPTION	APPROXIMATE PROPORTION (%)
Trace	presence just detectable, little or no influence on soil properties	0-5
Some	presence easily detectable, little influence on soil properties	5-12

The USC group symbols should be included with each soil description as shown in Table E1.2.3

TABLE E1.2.3 - SOIL GROUP SYMBOLS

SOIL TYPE	PREFIX
Gravel	G
Sand	S
Silt	M
Clay	С
Organic	О
Peat	Pt

The group symbols are combined with qualifiers which indicate grading, plasticity or secondary components as shown on Table E1.2.4

TABLE E1.2.4 - SOIL GROUP QUALIFIERS

SUBGROUP	SUFFIX
Well graded	W
Poorly Graded	P
Silty	M
Clayey	C
Liquid Limit <50% - low to medium plasticity	L
Liquid Limit >50% - medium to high plasticity	Н

(b) Grading

"Well graded" Good representation of all

particle sizes from the largest to the smallest.

"Poorly graded" One or more intermediate

sizes poorly represented

"Gap graded" One or more intermediate

sizes absent

"Uniformly graded" Essentially single size

material.

(c) Particle shape and texture

The shape and surface texture of the coarse grained particles should be described.

Angularity may be expressed as "rounded", "subrounded", "sub-angular" or "angular".

Particle **form** can be "equidimensional", "flat" or elongate".

Surface texture can be "glassy", "smooth", "rough", pitted" or striated".

(d) Colour

The colour of the soil should be described in the moist condition using simple terms such as:

> Black White Grey Red Brown Orange Yellow Green Blue

These may be modified as necessary by "light" or "dark". Borderline colours may be described as a combination of two colours, eg red-brown.

For soils that contain more than one colour terms such as:

• Speckled Very small (<10 mm dia) patches

• Mottled Irregular

• Blotched Large irregular (>75 mm dia)

Streaked Randomly oriented streaks

(e) Minor Components

Secondary and minor components should be individually described in a similar manner to the dominant component.

E1.3 Soil Condition

(a) Moisture

Soil moisture condition is described as "dry", "moist" or "wet".

The moisture categories are defined as:

Dry (D) - Little or no moisture evident. Soils are running. Moist (M) - Darkened in colour with cool feel. Granular soil particles tend to adhere. No free water evident upon remoulding of cohesive soils.

In addition the moisture content of cohesive soils can be estimated in relation to their liquid or plastic limit.

(b) Consistency

Estimates of the consistency of a clay or silt soil may be made from manual examination, hand penetrometer test, SPT results or from laboratory tests to determine undrained shear or unconfined compressive strengths. The classification of consistency is defined in Table E1.3.1.

TABLE E1.3.1 - CONSISTENCY OF FINE-GRAINED SOILS

TERM	UNCONFINED	FIELD		
	STRENGTH	IDENTIFICATION		
	(kPa)			
Very Soft	<25	Easily penetrated by fist. Sample exudes between fingers when squeezed in the fist.		
Soft	25 - 50	Easily moulded in fingers. Easily penetrated 50 mm by thumb.		
Firm	50 - 100	Can be moulded by strong pressure in the fingers. Penetrated only with great effort.		
Stiff	100 - 200	Cannot be moulded in fingers. Indented by thumb but penetrated only with great effort.		
Very Stiff	200 - 400	Very tough. Difficult to cut with knife. Readily indented with thumb nail.		
Hard	>400	Brittle, can just be scratched with thumb nail. Tends to break into fragments.		

Unconfined compressive strength as derived by a hand penetrometer can be taken as approximately double the undrained shear strength $(q_u=2\ c_u)$.

(c) Density Index

The insitu density index of granular soils can be assessed from the results of SPT or cone penetrometer tests. Density index should not be estimated visually.

TABLE E1.3.2 - DENSITY OF GRANULAR SOILS

TERM	SPT N	STATIC	DENSITY
	VALUE	CONE	INDEX
		VALUE	(%)
		qc (MPa)	
Very Loose	0 - 3	0 - 2	0 - 15
Loose	3 - 8	2 - 5	15 - 35
Medium Dense	8 - 25	5 - 15	35 - 65
Dense	25 - 42	15 - 20	65 - 85
Very Dense	>42	>20	>85

E1.4 Soil Structure

(a) Zoning

A sample may consist of several zones differing in colour, grain size or other properties. Terms to classify these zones are:

Layer - continuous across exposure or sample

Lens - discontinuous with lenticular shape

Pocket - irregular inclusion

Each zone should be described, their distinguishing features, and the nature of the interzone boundaries.

(b) Defects

Defects which are present in the sample can include:

- fissures
- roots (containing organic matter)
- tubes (hollow)
- casts (infilled)

Defects should be described giving details of dimensions and frequency. Fissure orientation, planarity, surface condition and infilling should be noted. If there is a tendency to break into blocks, block dimensions should be recorded

E1.5 Soil Origin

Information which may be interpretative but which may contribute to the usefulness of the material description should be included. The most common interpreted feature is the origin of the soil. The assessment of the probable origin is based on the soil material description, soil structure and its relationship to other soil and rock materials.

Common terms used are:

"Residual Soil" - Material which appears to have been derived by weathering from the underlying rock. There is no evidence of transport.

"Colluvium" - Material which appears to have been transported from its original location. The method of movement is usually the combination of gravity and erosion.

"Landslide Debris" - An extreme form of colluvium where the soil has been transported by mass movement. The material is obviously distributed and contains distinct defects related to the slope failure.

"Alluvium" - Material which has been transported essentially by water. usually associated with former stream activity.

"Fill" - Material which has been transported and placed by man. This can range from natural soils which have been placed in a controlled manner in engineering construction to dumped waste material. A description of the constituents should include an assessment of the method of placement.

E1.6 Fine Grained Soils

The physical properties of fine grained soils are dominated by silts and clavs.

The definition of clay and silt soils is governed by their Atterberg Limits. Clay soils are characterised by the properties of cohesion and plasticity with cohesion defines as the ability to deform without rupture. Silts exhibit cohesion but have low plasticity or are non-plastic.

The field characteristics of clay soils include:

- dry lumps have appreciable dry strength and cannot be powdered
- volume changes occur with moisture content variation
- feels smooth when moist with a greasy appearance when cut.

The field characteristics of silt soils include:

- dry lumps have negligible dry strength and can be powdered easily
- dilatancy an increase in volume due to shearing is indicted by the presence of a shiny film of water after a hand sample is shaken. The water disappears upon remoulding. Very fine grained sands may also exhibit dilatancy.
- low plasticity index
- · feels gritty to the teeth

E1.7 Organic Soils

Organic soils are distinguished from other soils by their appreciable content of vegetable matter, usually derived from plant remains.

The soil usually has a distinctive smell and low bulk density.

The USC system uses the symbol Pt for partly decomposed organic material. The O symbol is combined with suffixes "O" or "H" depending on plasticity.

Where roots or root fibres are present their frequency and the depth to which they are encountered should be recorded. The presence of roots or root fibres does not necessarily mean the material is an "organic material" by classification.

Coal and lignite should be described as such and not simply as organic matter.



APPENDIX B - LABORATORY TEST RESULTS

14/1 Cowpasture Place, Wetherill Park NSW 2164

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Shrink Swell Index Report

Project: No.55 Martin Road, Badgerys Creek Project No.: 21649 Client: AMJ Demolition and Excavation P/L Report No.: 17/3920 Address: No.44 Pearson Street, South Wentworthville 2145 Report Date: 18/12/2017

Test Method: AS 1289.7.1.1 Page: 1 of 1

Sampling Procedure: AS 1289.1.3.1 Clause 3.1.3.2 - Thin Walled Sampler

STS / Sample No.		8653C/1	8653C/2	8653C/3		
Sam	ple Location	Borehole 6 Refer to Drawing	Borehole 7 Refer to Drawing	Borehole 15 Refer to Drawing		
Material Description		SILTY CLAY: light brown with light grey, trace of gravel	SILTY CLAY: light brown with light grey, trace of gravel	SILTY CLAY: orange brown with light grey		
С	Depth (m)	0.7 - 1.0	0.6 - 0.9	0.5 - 0.8		
Sa	ample Date	12/12/2017	12/12/2017	12/12/2017		
	Moisture Content (%)	16.3	10.6	15.2		
Shrink	Soil Crumbling	Nil	Nil	Nil		
Shr	Extent of Cracking	Fine Cracks	Open Cracks	Open Cracks		
	Strain (%)	2.5	1.8	3.1		
	Moisture Content Initial (%)	14.0	10.0	16.2		
Swell	Moisture Content Final (%)	34.7	20.0	35.3		
	Strain (%)	1.7	3.1	0.0		
Inert I	Inclusions (%)	<5	<10	<5		
Shrink Swell Index (%)		1.9	1.8	1.7		

Remarks:



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Orlando Mendoza

Technician: NP Orlando Mendoza - Laboratory Manager

Form: RPS41 Date of Issue: 01/07/15 Revision: 5

14/1 Cowpasture Place, Wetherill Park NSW 2164

Phone: (02)9756 2166 Fax: (02)9756 1137 Email: enquiries@smectesting.com.au



California Bearing Ratio Determination Report

Project: 55 MARTIN ROAD, BADGERYS CREEK Project No.: 21649 Client: AMJ Demolition and Excavation P/L Report No.: 17/3960 Address: No.44 Pearson Street, South Wentworthville 2145 Report Date: 20/12/2017

Test Method: AS 1289.6.1.1, 2.1.1

Page: 1 of 1 Compactive Effort: Standard No. of Days Soaked: 4

Target Compaction (%): 100 Surcharge (Kg): 4.5

Sampling Procedure: AS 1289.1.2.1 Clause 6.5.3 - Power Auger Drilling (Not covered under NATA Scope of Accreditation)

mple No.	8653C/1	8653C/2	8653C/3	8653C/4		
Location	Borehole 2 Refer to Drawing No. 17/3905	Borehole 4 Refer to Drawing No. 17/3905	Borehole 8 Refer to Drawing No. 17/3905	Borehole 17 Refer to Drawing No. 17/3905		
escription	Silty Gravelly Clay, red brown	Silty Clay, orange brown/light grey/light brown, trace of gravel	Slity Clay: light grey with yellow brown/orange brown	Silty Gravelly Clay, red brown		
Sample (m)	0.5-1.1	1.0-1.4	0.3-0.9	0.4-1.0		
e Date	13/12/2017	13/12/2017	13/12/2017	13/12/2017		
n Wet Basis m (%)	0.0	0.0	0.0	0.0		
ure Content	19	12.4	13.1	13		
Moisture nt (%)	22.9	20.5	17.3	17		
Dry Density m³)	1.648	1.581	1.691	1.74		
Before Soaking	1.65	1.582	1.679	1.743		
After Soaking	1.641	1.515	1.606	1.672		
Before Soaking	100.1	100.1	99.3	100.2		
After Soaking	99.6	95.9	94.9	96.1		
Before Soaking	22.7	20.0	17.6	16.9		
After Soaking	25.6	26.1	23	21.5		
atio Before	99	98	101.8	99.3		
Top 30mm	27.0	30.2	27.9	27.9		
Entire Depth	24.5	24.2	25.2	25.2		
Soaking (%)	0.6	4.4	4.6	4.3		
alue (%)	1.5	2.5	1.5	1.5		
ion (mm)	2.0	2.5	2.5	2.5		
	Location escription ample (m) e Date n Wet Basis m (%) ure Content 6) Moisture nt (%) Dry Density n³) Before Soaking After Soaking After Soaking After Soaking After Soaking Top 30mm Entire Depth Soaking (%)	Borehole 2 Refer to Drawing No. 17/3905 Bescription Silty Gravelly Clay, red brown ample (m) 0.5-1.1 Page Date 13/12/2017 Net Basis m (%) Ine Content holisture nt (%) Dry Density n³) Before Soaking After Soa	Borehole 2 Refer to Drawing No. 17/3905 Silty Clay, orange brown/light grey/light brown, trace of gravel	Borehole 2 Refer to Drawing No. 17/3905 Refer to Drawing Refer to Drawing No. 17/3905 Refer to Drawing Slity Clay: light grey with yellow brown/orange No. 17/300 No. 1	Borehole 2 Refer to Drawing No. 17/3905 Refer to Drawing Refer to Drawing No. 17/3905 Refer to Drawing Refer Refer	Borehole 2 Refer to Drawing No. 17/3905 Refer to Drawing

Remarks: +19mm material excluded from test

Technician: NP

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Form: RPS25 Date of Issue: 01/06/15 Revision: 12

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Phone: (02)9756 2166 Fax: (02)9756 1137 Email: enquiries@smectesting.com.au



Particle Size Distribution

Project: 55 MARTIN ROAD, BADGERYS CREEK

STS / Sample No.: 8653C/1

Client: AMJ Demolition and Excavation P/L

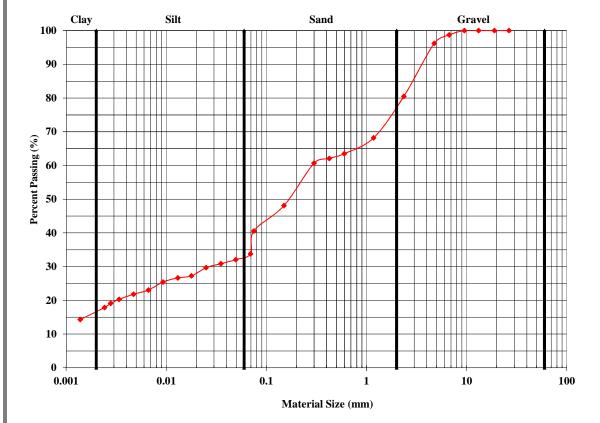
Sample Location: Borehole 13

Address: No.44 Pearson Street, South Wentworthville 2145 Depth (m): 0.0 - 0.4

Test Method: AS 1289.3.6.3 Method of Despersion: Mechanical Stirrer

Sampling Procedure: AS 1289.1.2.1 Clause 6.5.3 - Power Auger Drilling (Not covered under NATA Scope of Accreditation)

Material Description: Sand, brown, with clay/gravel, trace of silt



Remarks:

Technician: BV

NATA

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national standards
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Orlando Mendoza - Laboratory Manager

Project No.: 21649

Report No.: 17/3969

Report Date: 21/12/2017

Page: 1 of 2

Client Project No: N/A

Sieve Size (mm)	Percent Passing (%)
26.5	100
19.0	100
13.2	100
9.5	100
6.7	98.7
4.75	96.2
2.36	80.5
1.18	68.2
0.60	63.5
0.425	62.1
0.30	60.7
0.15	48.1
0.075	40.6
*Particle Size (mm)	Percent Passing (%)
0.0696	33.8
0.0496	32.1
0.0352	30.9
0.0250	29.7
0.0179	27.3
0.0131	26.7
0.0093	25.5
0.0066	23.0
0.0047	21.8
0.0034	20.3
0.0028	19.1
0.0024	17.9
0.0014	14.3

*Particle Size obtained by Hydrometer Analysis. Hydrometer Type: g/L

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Particle Size Distribution

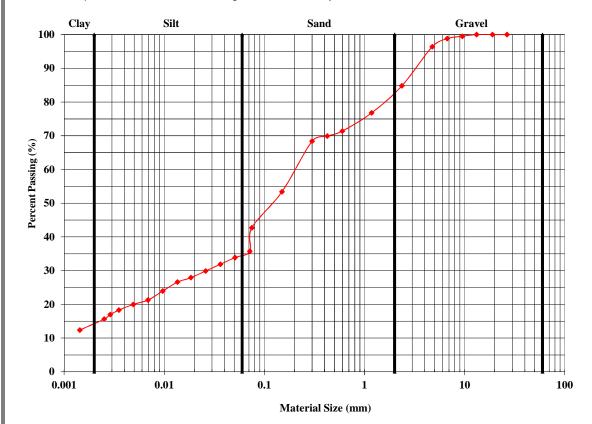
Project: 55 MARTIN ROAD, BADGERYS CREEK STS / Sample No.: 8653C/2 Client: AMJ Demolition and Excavation P/L Sample Location: Borehole 14

Address: No.44 Pearson Street, South Wentworthville 2145 Depth (m): 0.0 - 0.4

Method of Despersion: Mechanical Stirrer Test Method: AS 1289.3.6.3

Sampling Procedure: AS 1289.1.2.1 Clause 6.5.3 - Power Auger Drilling (Not covered under NATA Scope of Accreditation)

Material Description: Sand, brown, with silt/gravel, trace of clay



Remarks:

Technician: BV

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Orlando Mendoza - Laboratory Manager

Project No.: 21649

Report No.: 17/3969

Report Date: 21/12/2017

Page: 2 OF 2

Client Project No: N/A

Sieve Size (mm)	Percent Passing (%)		
26.5	100		
19.0	100		
13.2	100		
9.5	99.5		
6.7	98.8		
4.75	96.4		
2.36	84.8		
1.18	76.8		
0.60	71.4		
0.425	69.9		
0.30	68.4		
0.15	53.4		
0.075	42.7		
*Particle Size (mm)			
0.0717	35.7		
0.0510	33.9		
0.0364	31.9		
0.0259	29.9		
0.0185	27.9		
0.0135	26.6		
0.0097	24.0		
0.0069	21.3		
0.0049	20.0		
0.0035	18.3		
0.0029	17.0		
0.0025	15.7		
0.0014	12.4		

*Particle Size obtained by Hydrometer Analysis. Hydrometer Type: g/L

Form: RPS15b Date of Issue: 01/06/15 Revision: 9

14/1 Cowpasture Place, Wetherill Park NSW 2164

Phone: (02)9756 2166 Fax: (02)9756 1137 Email: enquiries@smectesting.com.au



Emerson Class No.

Project: NO.6 EDWARD STREET, NELSON

Client: THE SALVATION ARMY PROPERTY TRUST

Address: 265 CHALMERS STREET, REDFERN NSW 2016

Project No.: 21825

Report No.: 18/0101

Report Date: 16/01/2018

Test Method: AS 1289.3.8.1 Page: 1 OF 1

Sampling Procedure: AS 1289.1.2.1 Clause 6.5.3 - Power Auger Drilling (Not covered under NATA Scope of Accreditation)

1					
STS / Sample No.	8653C/1	8653C/2	8653C/3	8653C/4	
Sample Location	Borehole 2	Borehole 4	Borehole 8	Borehole 17	
Material Description	SILTY CLAY: red brown with orange brown and light grey	SILTY CLAY: orange brown with light grey and some light brown, trace of fine grained sand	SILTY CLAY: orange brown with light grey	SILTY CLAY: orange brown with light grey	
Depth (mm)	0.5 - 1.1	1.0 - 1.4	0.3 - 0.9	0.4 - 1.0	
Sample Date	12/12/2017	12/12/2017	12/12/2017	12/12/2017	
Date Tested	11/01/2018	11/01/2018	11/01/2018	11/01/2018	
Source of Material	Disturbed	Disturbed	Disturbed	Disturbed	
Water Temperature (°)	20	20	20	20	
Emerson Class No.	6	5	3	3	

Emerson Classification

Class 1: Slaking and complete dispersion before remoulding

Class 2: Slaking and some dispersion before remoulding

Class 3: Slaking and no dispersion before remoulding, dispersion after remoulding

Class 4: Slaking and no despersion before remoulding, no dispersion after remoulding, calcite or gypsum present

Class 5: Slaking and no dispersion before remoulding, no dispersion after remoulding, no calcite or gypsum present, dispersion after slaking in a 1:5 soil / water suspension

Class 6: Slaking and no dispersion before remoulding, no dispersion after remoulding, no calcite or gypsum present, flocculation after shaking in a 1:5 soil / water suspension

Class 7: No slaking, swelling occurs

F۷

Class 8: No slaking, swelling does not occur

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Form: RPS17 Date of Issue: 01/06/15 Revision: 7



CERTIFICATE OF ANALYSIS

Work Order : ES1731937

: SMEC TESTING SERVICES PTY LTD

Contact : SMEC TESTING ALL RESULTS

Address : P O BOX 6989

WETHERILL PARK NSW, AUSTRALIA 2164

Telephone : ---Project : 21649
Order number : E-2017-713

C-O-C number : ---Sampler : ---Site : ---Quote number : ---No. of samples received : 24
No. of samples analysed : 18

Page : 1 of 15

Laboratory : Environmental Division Sydney

Contact : Customer Services ES

Address : 277-289 Woodpark Road Smithfield NSW Australia 2164

Telephone : +61-2-8784 8555

Date Samples Received : 14-Dec-2017 16:02

Date Analysis Commenced : 19-Dec-2017

Issue Date : 27-Dec-2017 13:42



This report supersedes any previous report(s) with this reference. Results apply to the sample(s) as submitted. This document shall not be reproduced, except in full.

This Certificate of Analysis contains the following information:

- General Comments
- Analytical Results
- Descriptive Results
- Surrogate Control Limits

Additional information pertinent to this report will be found in the following separate attachments: Quality Control Report, QA/QC Compliance Assessment to assist with Quality Review and Sample Receipt Notification.

Signatories

Client

This document has been electronically signed by the authorized signatories below. Electronic signing is carried out in compliance with procedures specified in 21 CFR Part 11.

Signatories	Position	Accreditation Category
Ankit Joshi	Inorganic Chemist	Sydney Inorganics, Smithfield, NSW
Dian Dao		Sydney Inorganics, Smithfield, NSW
Edwandy Fadjar	Organic Coordinator	Sydney Inorganics, Smithfield, NSW
Edwandy Fadjar	Organic Coordinator	Sydney Organics, Smithfield, NSW
Raymond Commodore	Instrument Chemist	Sydney Inorganics, Smithfield, NSW
Shaun Spooner	Asbestos Identifier	Newcastle - Asbestos, Mayfield West, NSW

Page : 2 of 15 Work Order : ES1731937

Client : SMEC TESTING SERVICES PTY LTD

Project : 21649

General Comments

The analytical procedures used by the Environmental Division have been developed from established internationally recognized procedures such as those published by the USEPA, APHA, AS and NEPM. In house developed procedures are employed in the absence of documented standards or by client request.

Where moisture determination has been performed, results are reported on a dry weight basis.

Where a reported less than (<) result is higher than the LOR, this may be due to primary sample extract/digestate dilution and/or insufficient sample for analysis.

Where the LOR of a reported result differs from standard LOR, this may be due to high moisture content, insufficient sample (reduced weight employed) or matrix interference.

When sampling time information is not provided by the client, sampling dates are shown without a time component. In these instances, the time component has been assumed by the laboratory for processing purposes.

Where a result is required to meet compliance limits the associated uncertainty must be considered. Refer to the ALS Contact for details.

Key: CAS Number = CAS registry number from database maintained by Chemical Abstracts Services. The Chemical Abstracts Service is a division of the American Chemical Society.

LOR = Limit of reporting

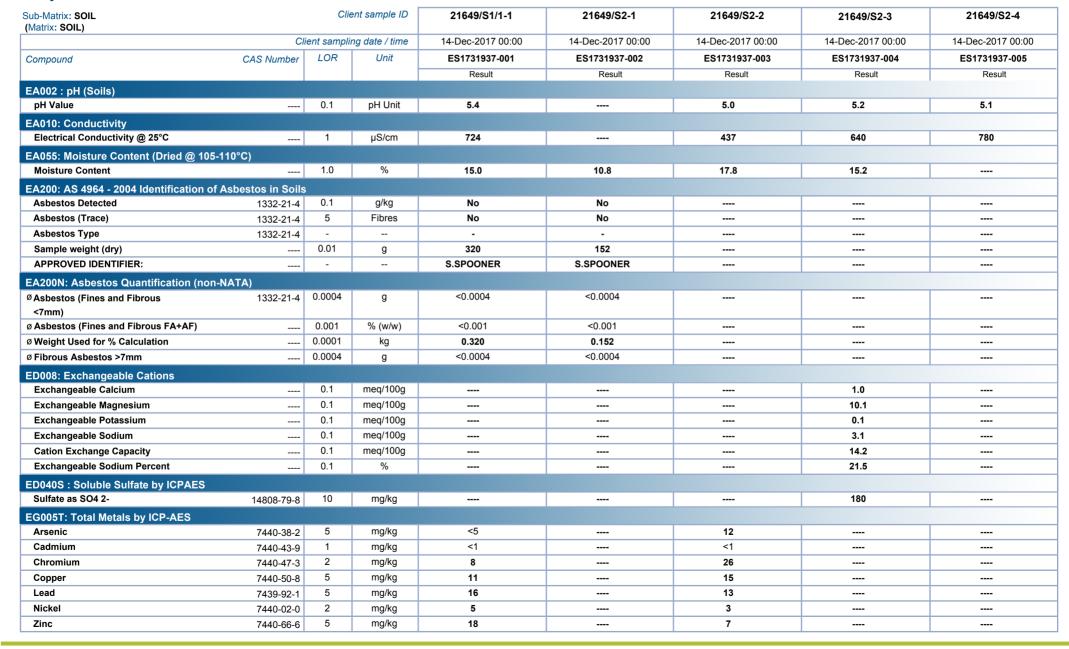
- ^ = This result is computed from individual analyte detections at or above the level of reporting
- ø = ALS is not NATA accredited for these tests.
- ~ = Indicates an estimated value.
- EA200N: Asbestos weights and percentages are not covered under the Scope of NATA Accreditation.
 - Weights of Asbestos are based on extracted bulk asbestos, fibre bundles, and/or ACM and do not include respirable fibres (if present)
 - The Asbestos (Fines and Fibrous) weight is calculated from the extracted Fibrous Asbestos and Asbestos Fines as an equivalent weight of 100% Asbestos
 - Percentages for Asbestos content in ACM are based on the 2013 NEPM default values.
- All calculations of percentage Asbestos under this method are approximate and should be used as a guide only.
- EA200 'Am' Amosite (brown asbestos)
- EA200 'Cr' Crocidolite (blue asbestos)
- EA200 'Trace' Asbestos fibres ("Free Fibres") detected by trace analysis per AS4964. The result can be interpreted that the sample contains detectable 'respirable' asbestos fibres
- EA200: Asbestos Identification Samples were analysed by Polarised Light Microscopy including dispersion staining.
- EA200 Legend
- EA200 'Ch' Chrysotile (white asbestos)
- EA200: 'UMF' Unknown Mineral Fibres. "-" indicates fibres detected may or may not be asbestos fibres. Confirmation by alternative techniques is recommended.
- EA200: Negative results for vinyl tiles should be confirmed by an independent analytical technique.
- EA200N: ALS laboratory procedures and methods used for the identification and quantitation of asbestos are consistent with AS4964-2004 and the requirements of the 2013 NEPM for Assessment of Site Contamination
- EA200: For samples larger than 30g, the <2mm fraction may be sub-sampled prior to trace analysis as outlined in ISO23909:2008(E) Sect 6.3.2-2
- ED007 and ED008: When Exchangeable Al is reported from these methods, it should be noted that Rayment & Lyons (2011) suggests Exchange Acidity by 1M KCI Method 15G1 (ED005) is a more suitable method for the determination of exchange acidity (H+ + Al3+).
- EA200: 'Yes' Asbestos detected by polarised light microscopy including dispersion staining.
- EA200: 'No*' No asbestos found, at the reporting limit of 0.1g/kg, by polarised light microscopy including dispersion staining. Asbestos material was detected and positively identified at concentrations estimated to be below 0.1g/kg.
- EA200: 'No' No asbestos found at the reporting limit 0.1g/kg, by polarised light microscopy including dispersion staining.



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Client : SMEC TESTING SERVICES PTY LTD

Project : 21649





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Client : SMEC TESTING SERVICES PTY LTD

Project : 21649

Analytical Results

Chlorpyrifos-methyl

5598-13-0

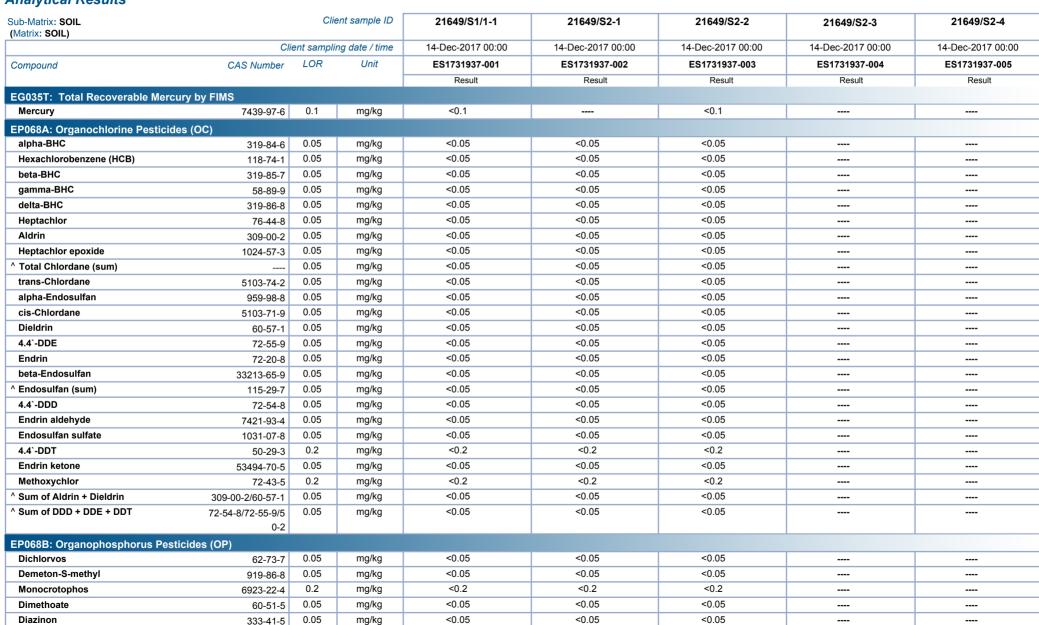
0.05

mg/kg

< 0.05

< 0.05

< 0.05

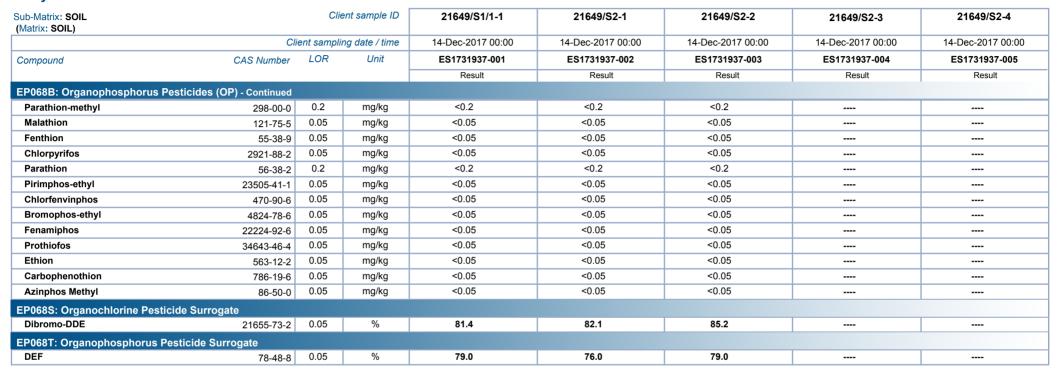




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Client : SMEC TESTING SERVICES PTY LTD

Project : 21649

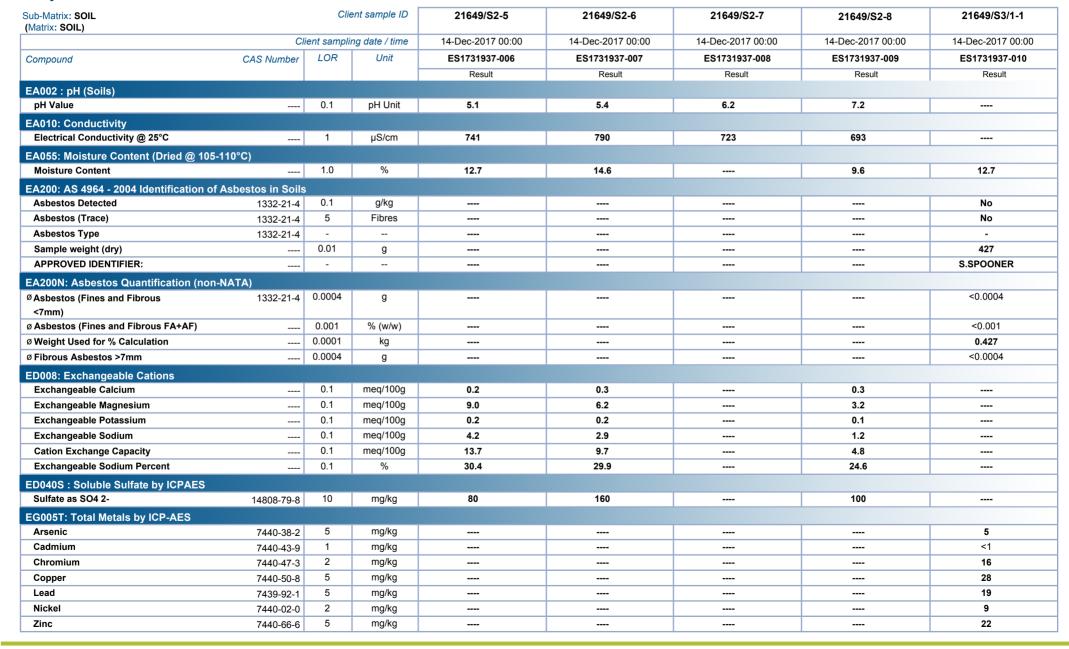




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Client : SMEC TESTING SERVICES PTY LTD

Project : 21649

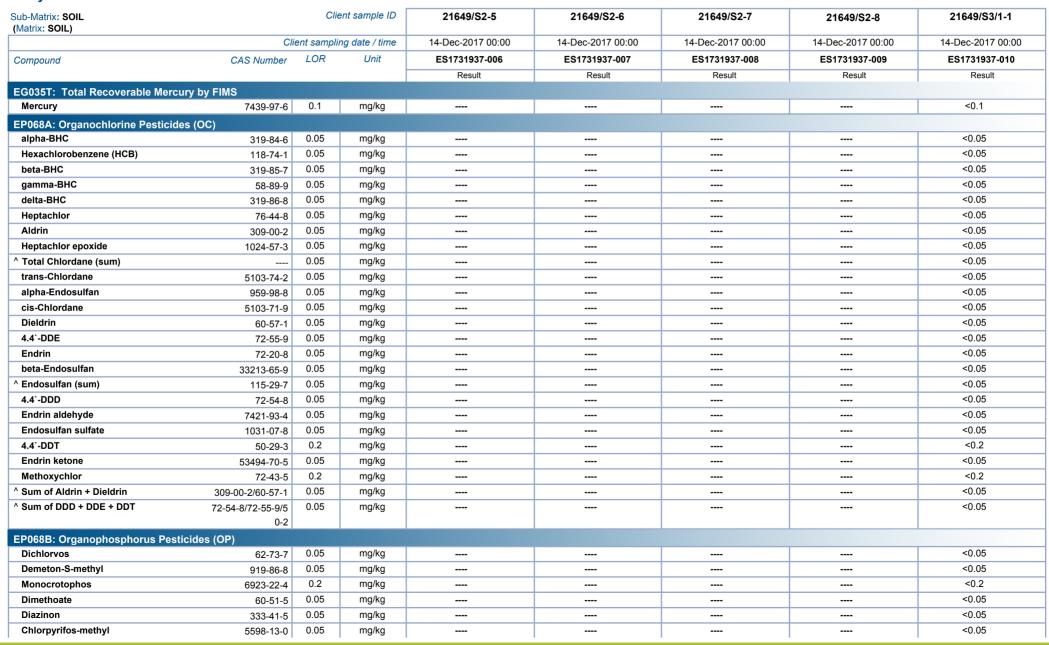




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Client : SMEC TESTING SERVICES PTY LTD

Project : 21649





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Client : SMEC TESTING SERVICES PTY LTD

Project : 21649

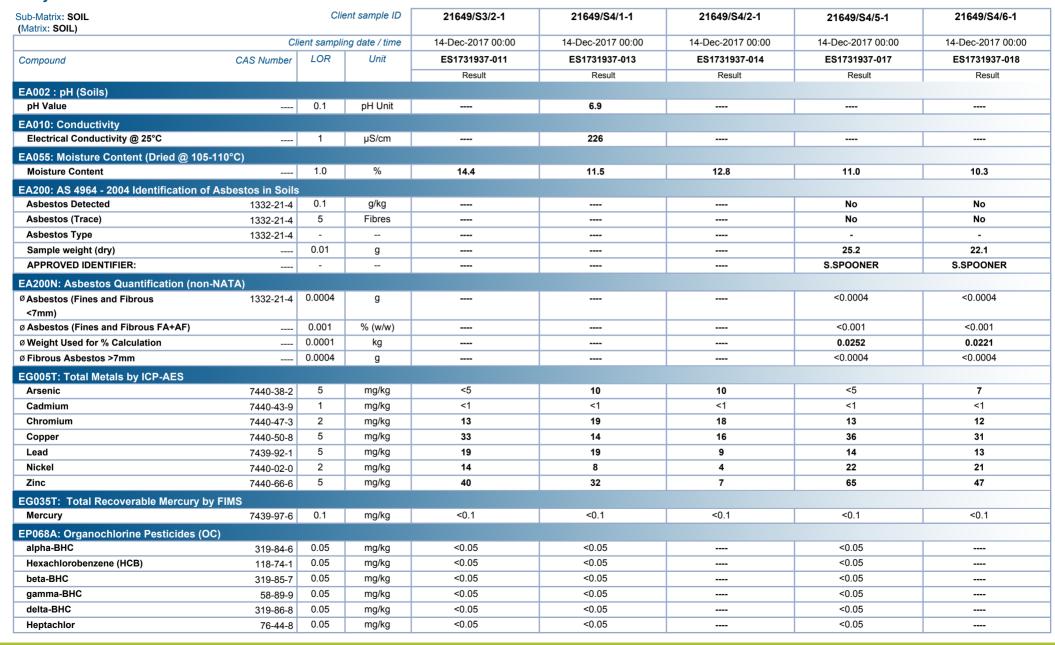




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Client : SMEC TESTING SERVICES PTY LTD

Project : 21649

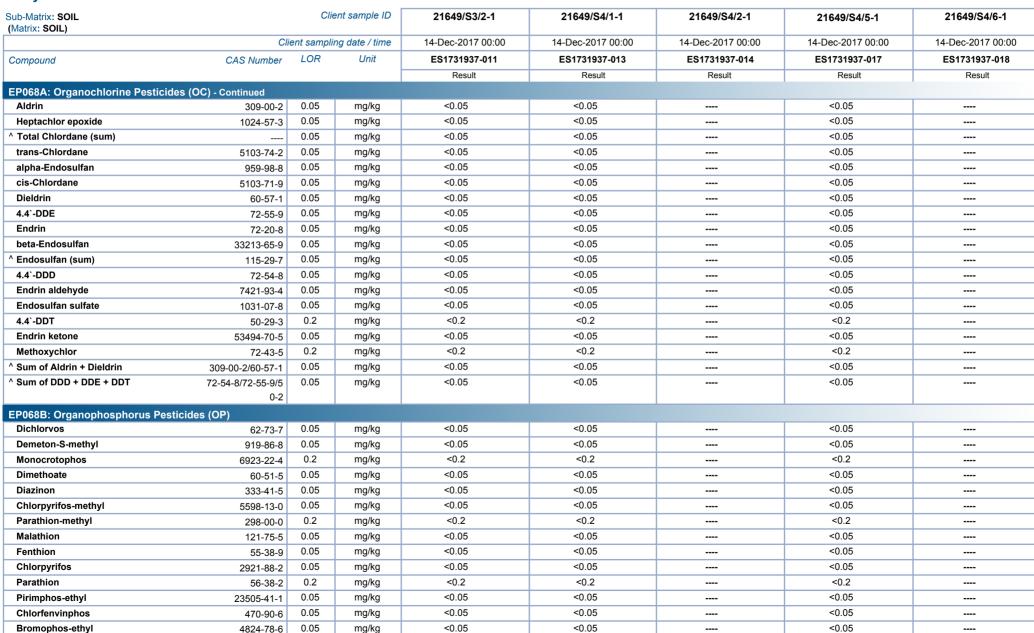




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Client : SMEC TESTING SERVICES PTY LTD

Project : 21649

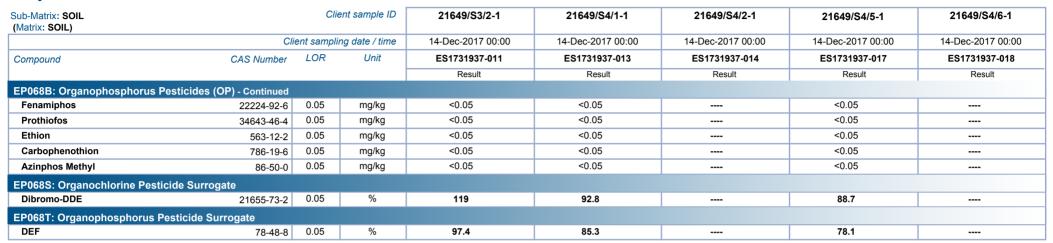




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Client : SMEC TESTING SERVICES PTY LTD

Project : 21649





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Client : SMEC TESTING SERVICES PTY LTD

Project : 21649



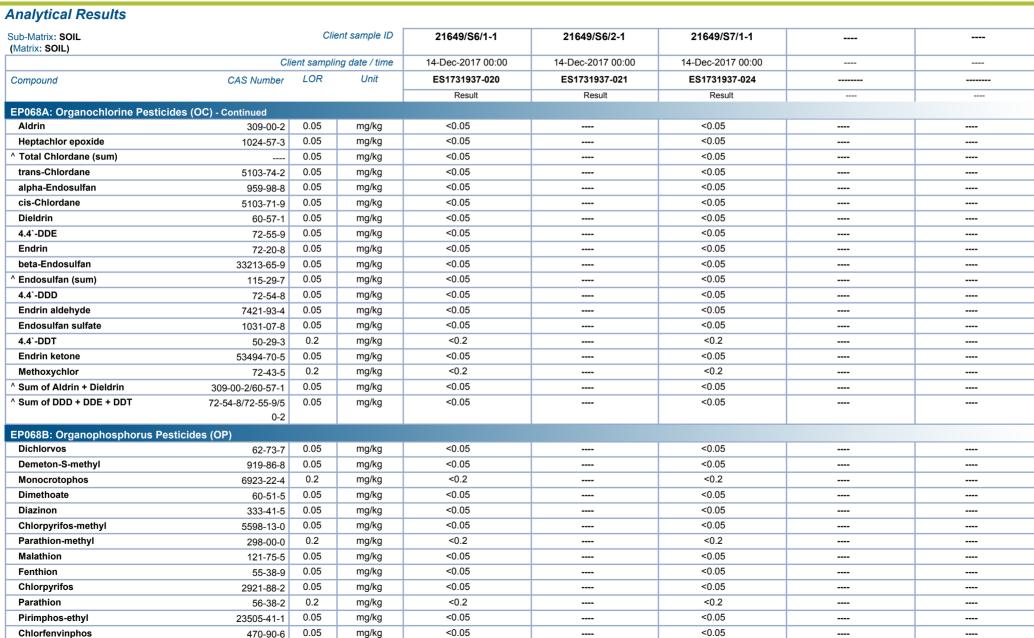
Sub-Matrix: SOIL (Matrix: SOIL)		Clie	ent sample ID	21649/S6/1-1	21649/S6/2-1	21649/\$7/1-1	
	Ci	lient samplii	ng date / time	14-Dec-2017 00:00	14-Dec-2017 00:00	14-Dec-2017 00:00	
Compound	CAS Number	LOR	Unit	ES1731937-020	ES1731937-021	ES1731937-024	
				Result	Result	Result	
A002 : pH (Soils)							
pH Value		0.1	pH Unit	7.0		6.8	
EA010: Conductivity							
Electrical Conductivity @ 25°C		1	μS/cm	52		84	
EA055: Moisture Content (Dried @ 105	·110°C)						
Moisture Content		1.0	%	11.3	13.2	10.6	
EA200: AS 4964 - 2004 Identification of							
Asbestos Detected	1332-21-4	0.1	g/kg	No		No	
Asbestos (Trace)	1332-21-4	5	Fibres	No		No	
Asbestos Type	1332-21-4	-		-		-	
Sample weight (dry)	1332-21-4	0.01	g	324		311	
APPROVED IDENTIFIER:		-		S.SPOONER		S.SPOONER	
				U.U. UUITEIN		U.U. CONEIX	
EA200N: Asbestos Quantification (non		0.0004		-0.0004		.0.0004	
Ø Asbestos (Fines and Fibrous	1332-21-4	0.0004	g	<0.0004		<0.0004	
<7mm)		0.004	0/ //)	40.004		10.004	
Ø Asbestos (Fines and Fibrous FA+AF)		0.001	% (w/w)	<0.001		<0.001	
Ø Weight Used for % Calculation		0.0001	kg	0.324		0.311	
Ø Fibrous Asbestos >7mm		0.0004	g	<0.0004		<0.0004	
EG005T: Total Metals by ICP-AES							
Arsenic	7440-38-2	5	mg/kg	7	10	8	
Cadmium	7440-43-9	1	mg/kg	<1	<1	<1	
Chromium	7440-47-3	2	mg/kg	19	16	16	
Copper	7440-50-8	5	mg/kg	25	44	15	
Lead	7439-92-1	5	mg/kg	18	17	14	
Nickel	7440-02-0	2	mg/kg	17	18	12	
Zinc	7440-66-6	5	mg/kg	38	50	26	
EG035T: Total Recoverable Mercury by	FIMS						
Mercury	7439-97-6	0.1	mg/kg	<0.1	<0.1	<0.1	
EP068A: Organochlorine Pesticides (O	C)						
alpha-BHC	319-84-6	0.05	mg/kg	<0.05		<0.05	
Hexachlorobenzene (HCB)	118-74-1	0.05	mg/kg	<0.05		<0.05	
beta-BHC	319-85-7	0.05	mg/kg	<0.05		<0.05	
gamma-BHC	58-89-9	0.05	mg/kg	<0.05		<0.05	
delta-BHC	319-86-8	0.05	mg/kg	<0.05		<0.05	
Heptachlor	76-44-8	0.05	mg/kg	<0.05		<0.05	

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Client : SMEC TESTING SERVICES PTY LTD

Project : 21649

Bromophos-ethyl



< 0.05

4824-78-6

0.05

mg/kg

< 0.05



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Client : SMEC TESTING SERVICES PTY LTD

Project : 21649

Analytical Results



Sub-Matrix: SOIL (Matrix: SOIL)		Clie	ent sample ID	21649/S6/1-1	21649/S6/2-1	21649/\$7/1-1	
	Cli	ent sampli	ng date / time	14-Dec-2017 00:00	14-Dec-2017 00:00	14-Dec-2017 00:00	
Compound	CAS Number	LOR	Unit	ES1731937-020	ES1731937-021	ES1731937-024	
				Result	Result	Result	
EP068B: Organophosphorus Pesticide	s (OP) - Continued						
Fenamiphos	22224-92-6	0.05	mg/kg	<0.05		<0.05	
Prothiofos	34643-46-4	0.05	mg/kg	<0.05		<0.05	
Ethion	563-12-2	0.05	mg/kg	<0.05		<0.05	
Carbophenothion	786-19-6	0.05	mg/kg	<0.05		<0.05	
Azinphos Methyl	86-50-0	0.05	mg/kg	<0.05		<0.05	
EP068S: Organochlorine Pesticide Sur	rogate						
Dibromo-DDE	21655-73-2	0.05	%	77.0		90.5	
EP068T: Organophosphorus Pesticide	Surrogate						
DEF	78-48-8	0.05	%	70.0		83.8	

Analytical Results Descriptive Results

Sub-Matrix: SOIL

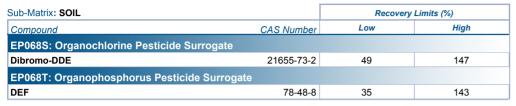
Method: Compound	Client sample ID - Client sampling date / time	Analytical Results
EA200: AS 4964 - 2004 Identification of Asbestos	in Soils	
EA200: Description	21649/S1/1-1 - 14-Dec-2017 00:00	Mid brown clay soil.
EA200: Description	21649/S2-1 - 14-Dec-2017 00:00	Mid brown clay soil.
EA200: Description	21649/S3/1-1 - 14-Dec-2017 00:00	Mid brown clay soil.
EA200: Description	21649/S4/5-1 - 14-Dec-2017 00:00	Mid brown clay soil.
EA200: Description	21649/S4/6-1 - 14-Dec-2017 00:00	Mid brown clay soil.
EA200: Description	21649/S6/1-1 - 14-Dec-2017 00:00	Mid brown clay soil.
EA200: Description	21649/S7/1-1 - 14-Dec-2017 00:00	Mid brown clay soil.

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Client : SMEC TESTING SERVICES PTY LTD

Project : 21649

Surrogate Control Limits







CERTIFICATE OF ANALYSIS

Work Order : **ES1731925**

Client : SMEC TESTING SERVICES PTY LTD

Contact : SMEC TESTING ALL RESULTS

Address : P O BOX 6989

WETHERILL PARK NSW, AUSTRALIA 2164

Telephone : ---Project : 21649
Order number : E-2017-713

 C-O-C number
 : ---

 Sampler
 : ---

 Site
 : ---

 Quote number
 : ---

 No. of samples received
 : 36

 No. of samples analysed
 : 33

Page : 1 of 24

Laboratory : Environmental Division Sydney

Contact : Customer Services ES

Address : 277-289 Woodpark Road Smithfield NSW Australia 2164

Telephone : +61-2-8784 8555

Date Samples Received : 14-Dec-2017 16:02

Date Analysis Commenced : 18-Dec-2017

Issue Date : 02-Jan-2018 17:24



This report supersedes any previous report(s) with this reference. Results apply to the sample(s) as submitted. This document shall not be reproduced, except in full.

This Certificate of Analysis contains the following information:

- General Comments
- Analytical Results
- Descriptive Results
- Surrogate Control Limits

Additional information pertinent to this report will be found in the following separate attachments: Quality Control Report, QA/QC Compliance Assessment to assist with Quality Review and Sample Receipt Notification.

Signatories

This document has been electronically signed by the authorized signatories below. Electronic signing is carried out in compliance with procedures specified in 21 CFR Part 11.

Signatories	Position	Accreditation Category
Ankit Joshi	Inorganic Chemist	Sydney Inorganics, Smithfield, NSW
Celine Conceicao	Senior Spectroscopist	Sydney Inorganics, Smithfield, NSW
Dian Dao		Sydney Inorganics, Smithfield, NSW
Edwandy Fadjar	Organic Coordinator	Sydney Inorganics, Smithfield, NSW
Edwandy Fadjar	Organic Coordinator	Sydney Organics, Smithfield, NSW
Greg Vogel	Laboratory Manager	Brisbane Inorganics, Stafford, QLD
Matt Frost	Senior Organic Chemist	Brisbane Inorganics, Stafford, QLD
Matt Frost	Senior Organic Chemist	Brisbane Organics, Stafford, QLD
Raymond Commodore	Instrument Chemist	Sydney Inorganics, Smithfield, NSW
Shaun Spooner	Asbestos Identifier	Newcastle - Asbestos, Mayfield West, NSW

Page : 2 of 24 Work Order : ES1731925

Client : SMEC TESTING SERVICES PTY LTD

Project : 21649

General Comments

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When sampling time information is not provided by the client, sampling dates are shown without a time component. In these instances, the time component has been assumed by the laboratory for processing purposes.

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Key: CAS Number = CAS registry number from database maintained by Chemical Abstracts Services. The Chemical Abstracts Service is a division of the American Chemical Society.

LOR = Limit of reporting

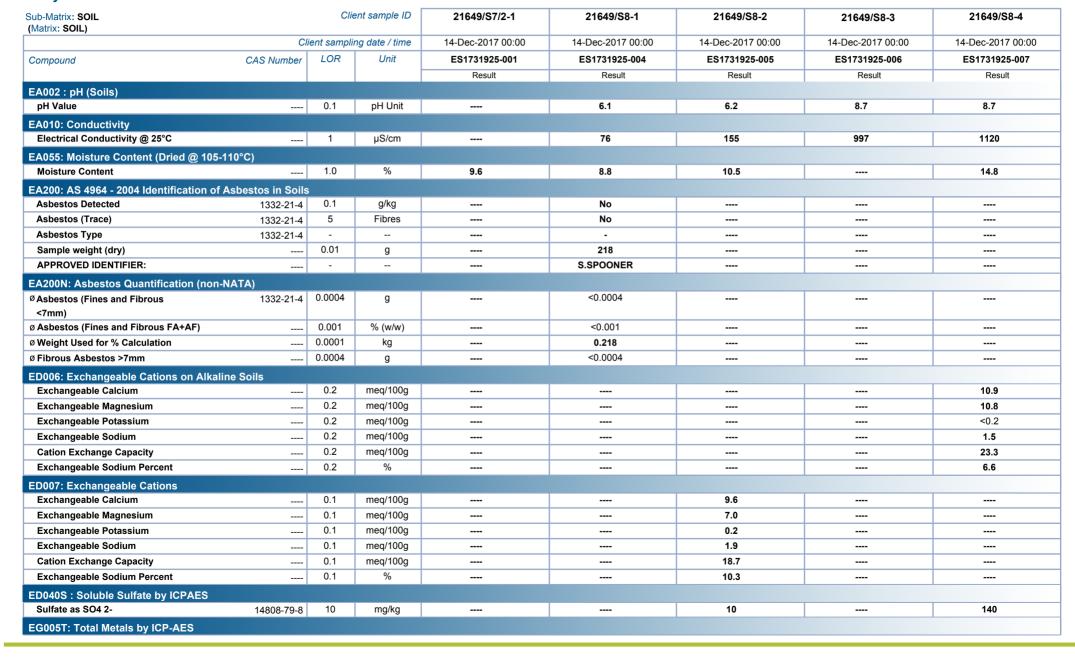
- ^ = This result is computed from individual analyte detections at or above the level of reporting
- ø = ALS is not NATA accredited for these tests
- ~ = Indicates an estimated value.
- EA200N: Asbestos weights and percentages are not covered under the Scope of NATA Accreditation.
 - Weights of Asbestos are based on extracted bulk asbestos, fibre bundles, and/or ACM and do not include respirable fibres (if present)
 - The Asbestos (Fines and Fibrous) weight is calculated from the extracted Fibrous Asbestos and Asbestos Fines as an equivalent weight of 100% Asbestos
 - Percentages for Asbestos content in ACM are based on the 2013 NEPM default values.
- All calculations of percentage Asbestos under this method are approximate and should be used as a guide only.
- EA200 'Am' Amosite (brown asbestos)
- EA200 'Cr' Crocidolite (blue asbestos)
- EA200 'Trace' Asbestos fibres ("Free Fibres") detected by trace analysis per AS4964. The result can be interpreted that the sample contains detectable 'respirable' asbestos fibres
- EA200: Asbestos Identification Samples were analysed by Polarised Light Microscopy including dispersion staining.
- EA200 Legend
- EA200 'Ch' Chrysotile (white asbestos)
- EA200: 'UMF' Unknown Mineral Fibres. "-" indicates fibres detected may or may not be asbestos fibres. Confirmation by alternative techniques is recommended.
- EA200: Negative results for vinyl tiles should be confirmed by an independent analytical technique.
- EA200N: ALS laboratory procedures and methods used for the identification and quantitation of asbestos are consistent with AS4964-2004 and the requirements of the 2013 NEPM for Assessment of Site Contamination
- Benzo(a)pyrene Toxicity Equivalent Quotient (TEQ) is the sum total of the concentration of the eight carcinogenic PAHs multiplied by their Toxicity Equivalence Factor (TEF) relative to Benzo(a)pyrene. TEF values are provided in brackets as follows: Benz(a)anthracene (0.1), Chrysene (0.01), Benzo(b+j) & Benzo(k)fluoranthene (0.1), Benzo(a)pyrene (1.0), Indeno(1.2.3.cd)pyrene (0.1), Dibenz(a.h)anthracene (1.0), Benzo(g.h.i)perylene (0.01). Less than LOR results for 'TEQ Zero' are treated as zero, for 'TEQ 1/2LOR' are treated as half the reported LOR, and for 'TEQ LOR' are treated as being equal to the reported LOR. Note: TEQ 1/2LOR and TEQ LOR will calculate as 0.6mg/Kg and 1.2mg/Kg respectively for samples with non-detects for all of the eight TEQ PAHs.
- EA200: For samples larger than 30g, the <2mm fraction may be sub-sampled prior to trace analysis as outlined in ISO23909:2008(E) Sect 6.3.2-2
- ED007 and ED008: When Exchangeable Al is reported from these methods, it should be noted that Rayment & Lyons (2011) suggests Exchange Acidity by 1M KCI Method 15G1 (ED005) is a more suitable method for the determination of exchange acidity (H+ + Al3+).
- EA200: 'Yes' Asbestos detected by polarised light microscopy including dispersion staining.
- EA200: 'No*' No asbestos found, at the reporting limit of 0.1g/kg, by polarised light microscopy including dispersion staining. Asbestos material was detected and positively identified at concentrations estimated to be below 0.1g/kg.
- EA200: 'No' No asbestos found at the reporting limit 0.1g/kg, by polarised light microscopy including dispersion staining.



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Client : SMEC TESTING SERVICES PTY LTD

Project : 21649

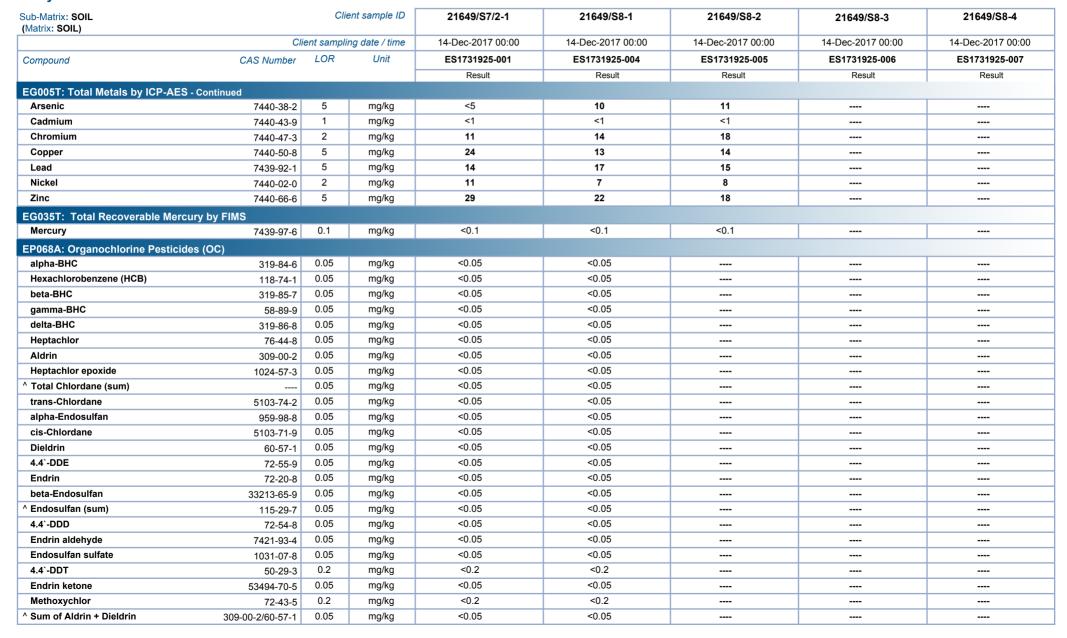




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Client : SMEC TESTING SERVICES PTY LTD

Project : 21649





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Client : SMEC TESTING SERVICES PTY LTD

Project : 21649

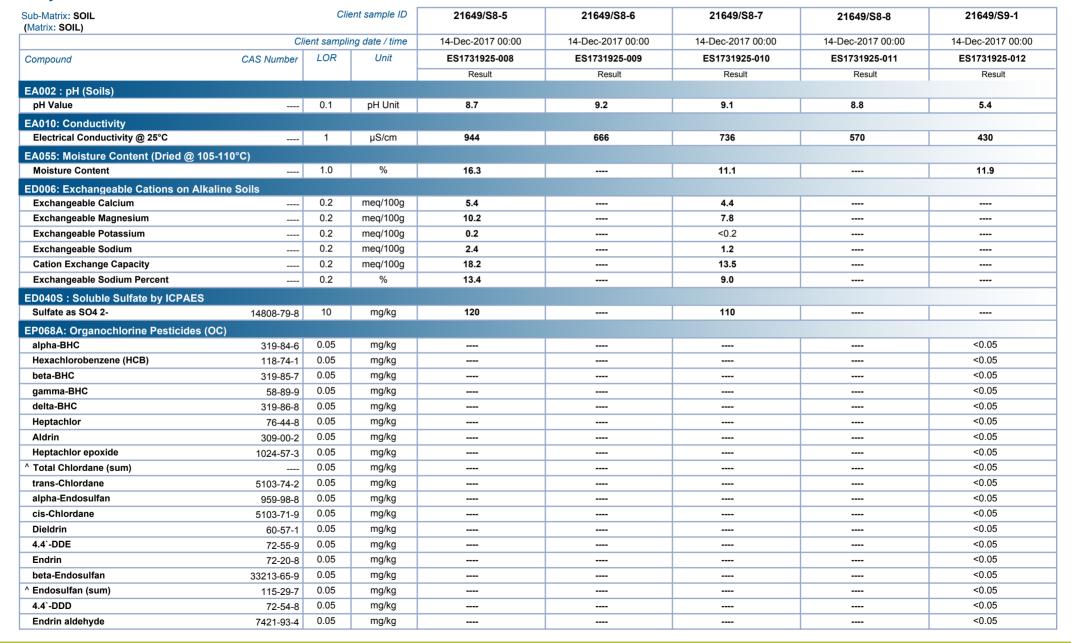




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Client : SMEC TESTING SERVICES PTY LTD

Project : 21649

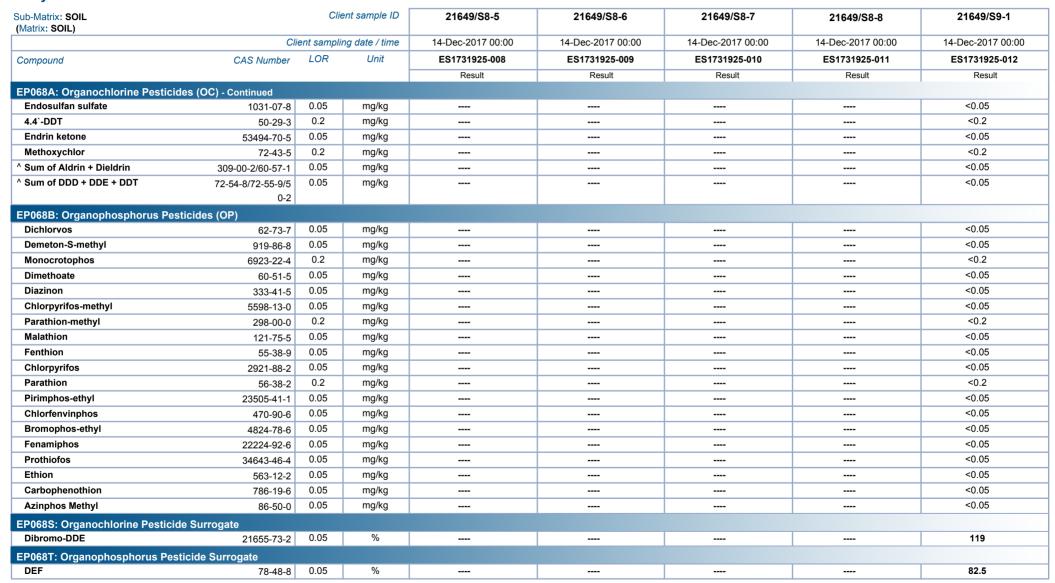




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Client : SMEC TESTING SERVICES PTY LTD

Project : 21649

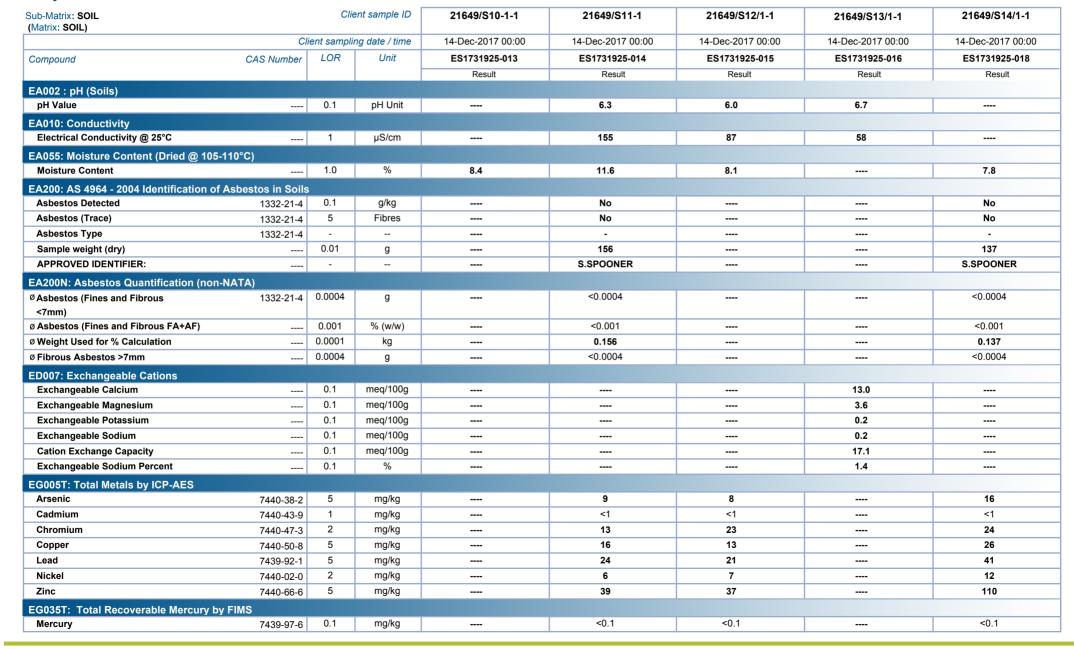




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Client : SMEC TESTING SERVICES PTY LTD

Project : 21649

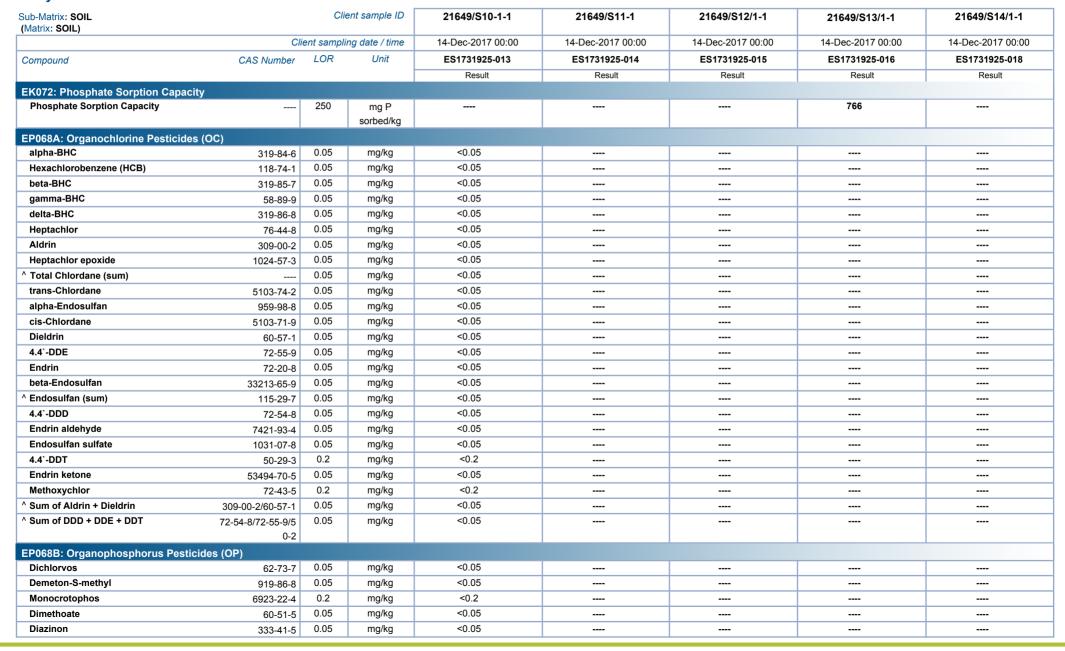




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Client : SMEC TESTING SERVICES PTY LTD

Project : 21649





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Client : SMEC TESTING SERVICES PTY LTD

0.05

78-48-8

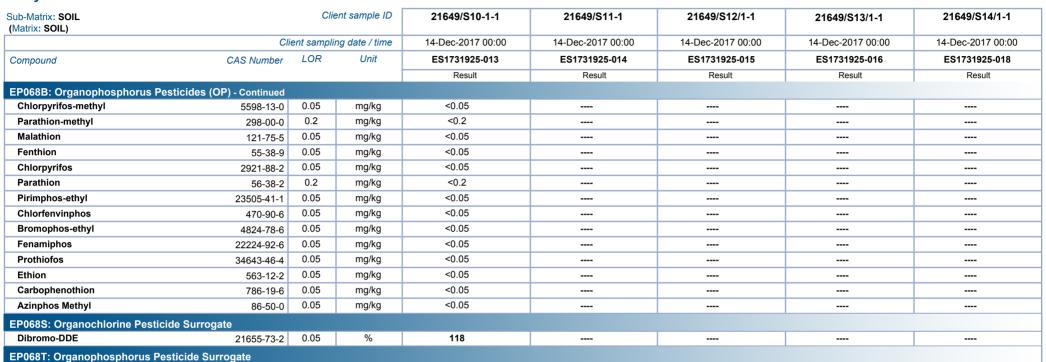
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83.9

Project : 21649

Analytical Results

DEF

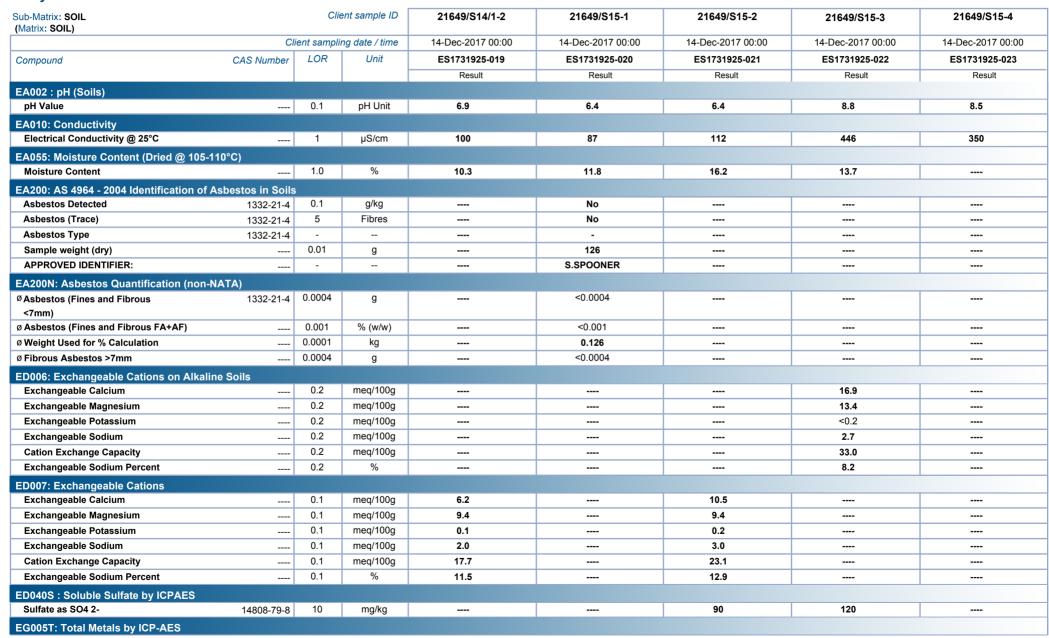




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Client : SMEC TESTING SERVICES PTY LTD

Project : 21649

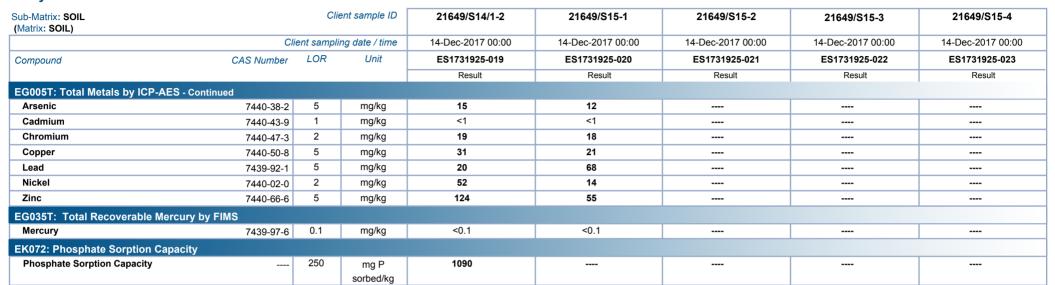




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Client : SMEC TESTING SERVICES PTY LTD

Project : 21649

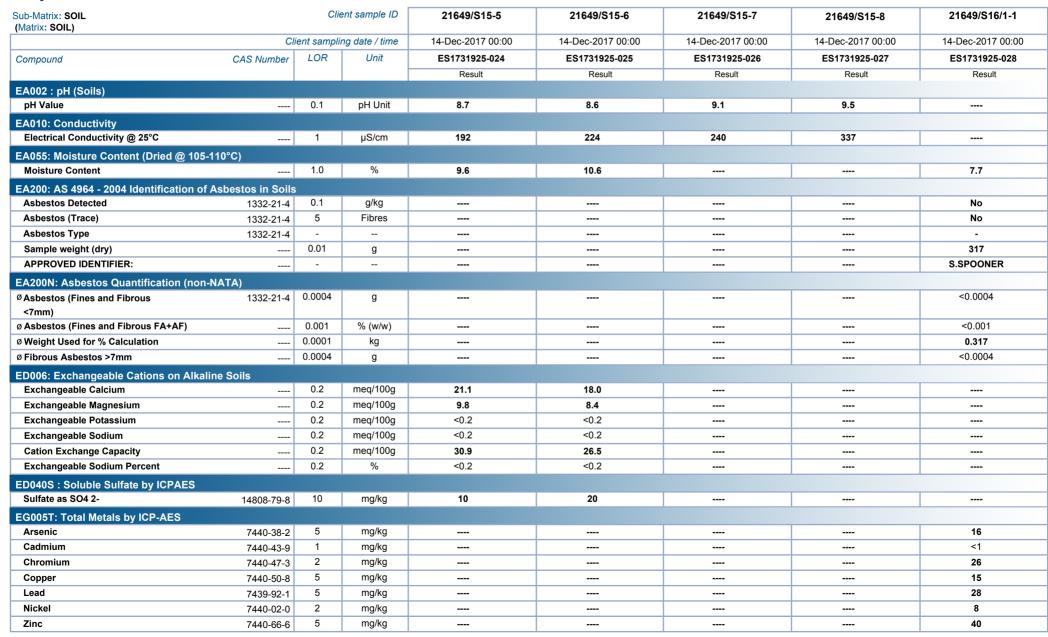




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Client : SMEC TESTING SERVICES PTY LTD

Project : 21649

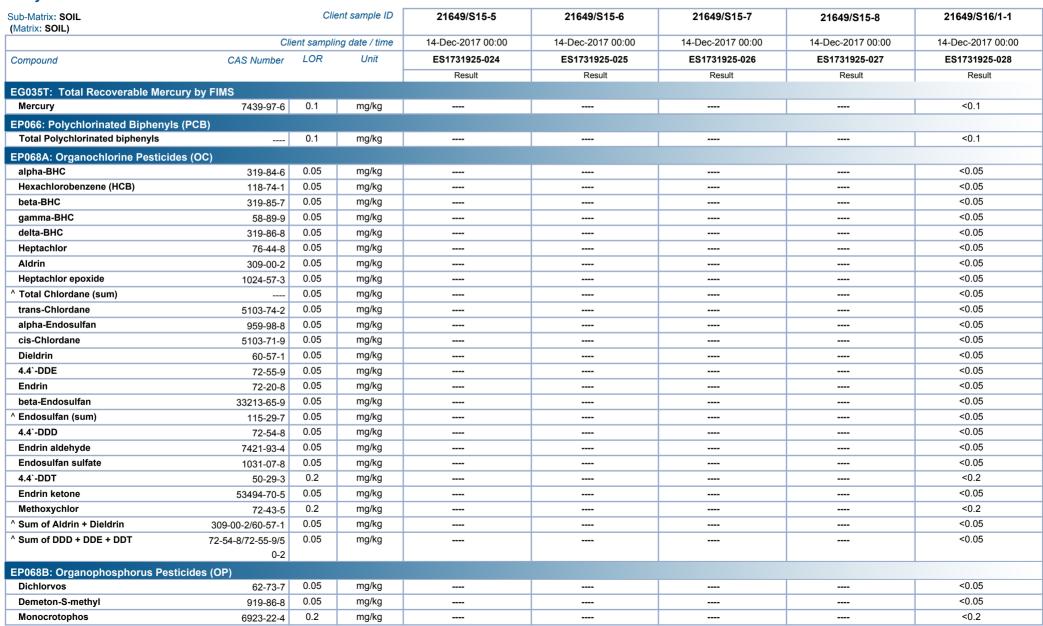




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Client : SMEC TESTING SERVICES PTY LTD

Project : 21649

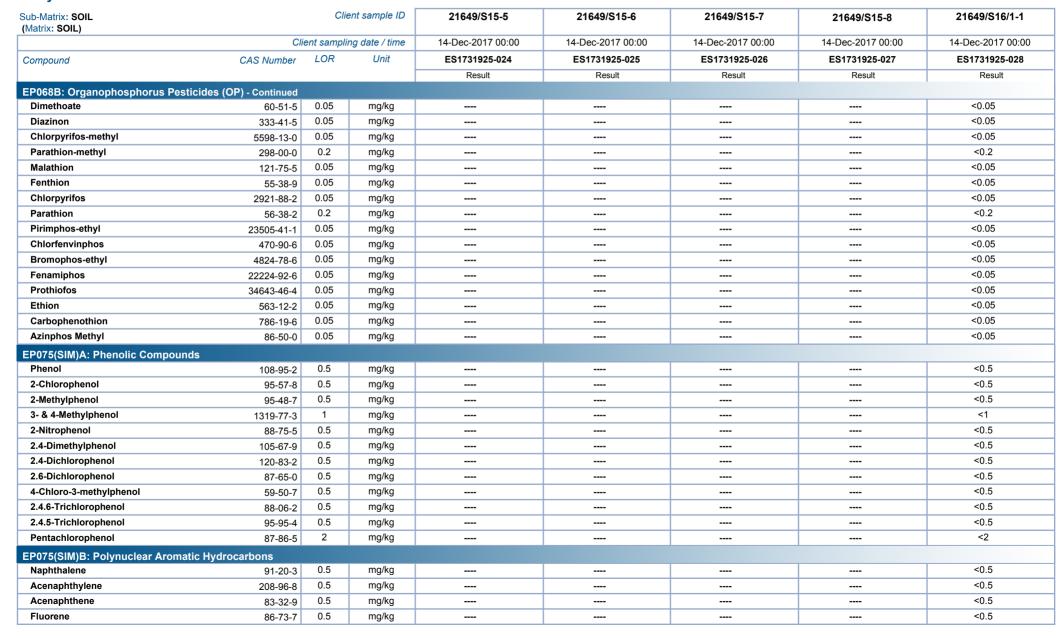




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Client : SMEC TESTING SERVICES PTY LTD

Project : 21649

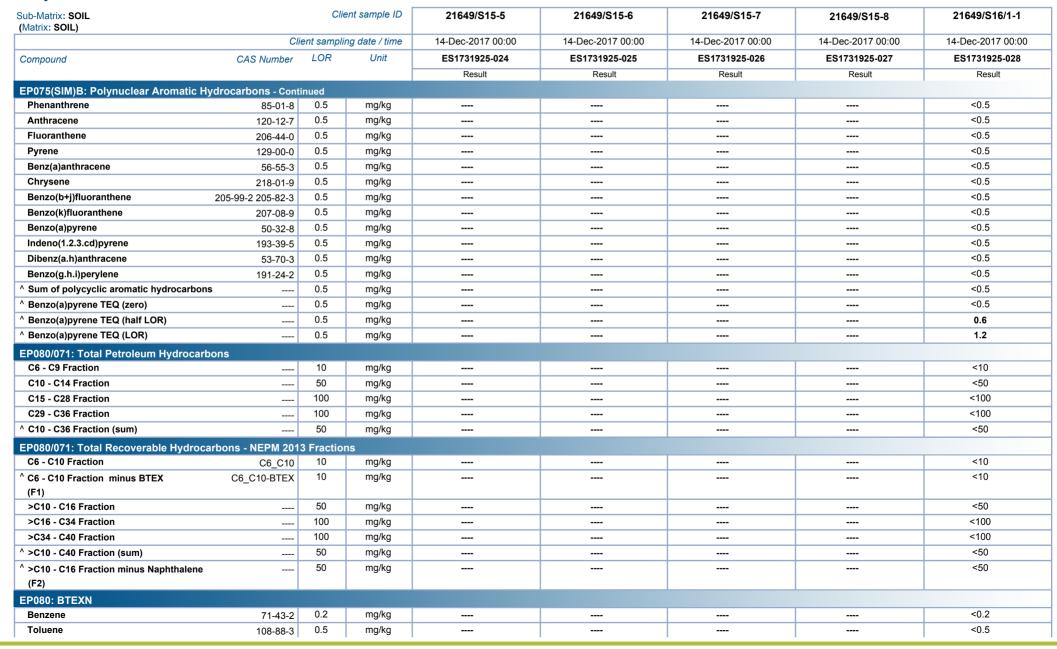




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Client : SMEC TESTING SERVICES PTY LTD

Project : 21649

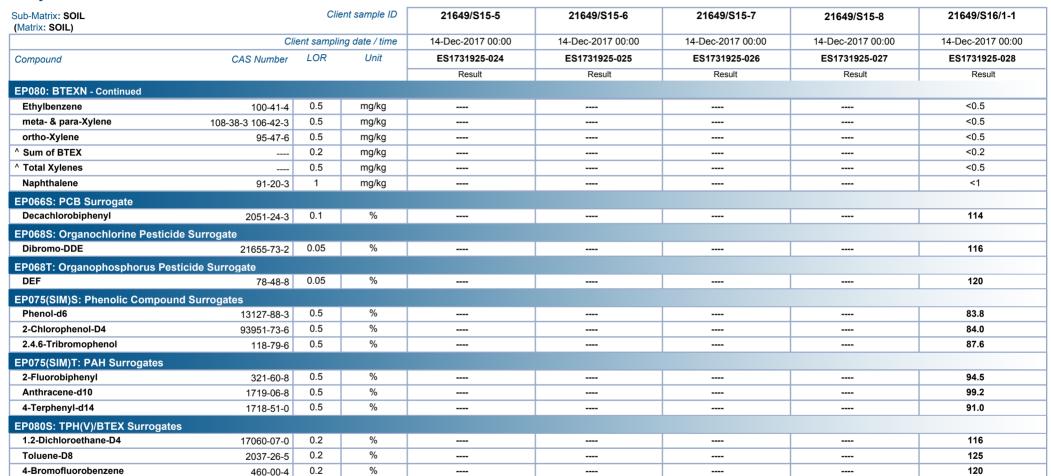




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Client : SMEC TESTING SERVICES PTY LTD

Project : 21649

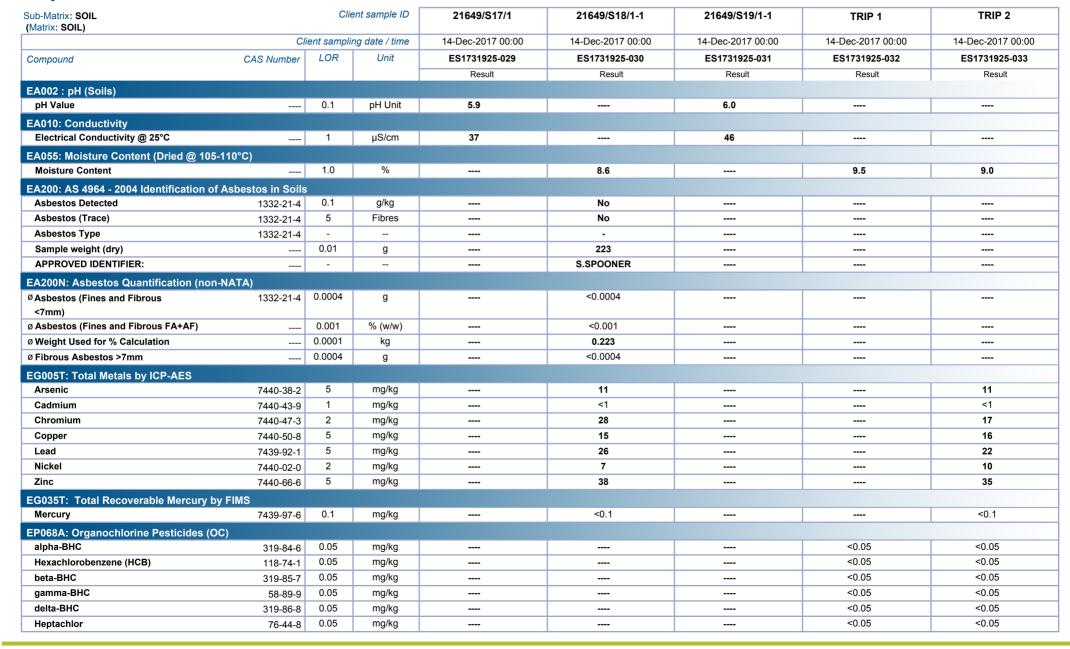




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Client : SMEC TESTING SERVICES PTY LTD

Project : 21649

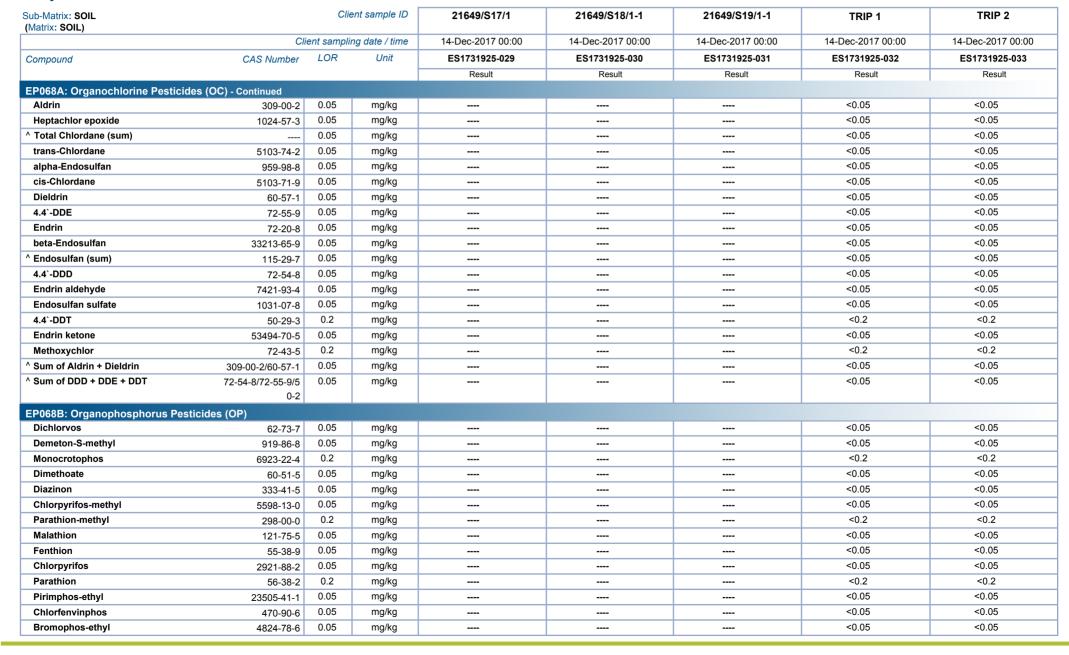




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Client : SMEC TESTING SERVICES PTY LTD

Project : 21649

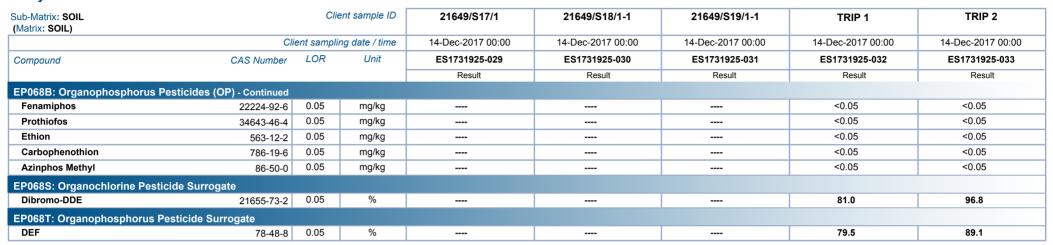




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Client : SMEC TESTING SERVICES PTY LTD

Project : 21649





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Client : SMEC TESTING SERVICES PTY LTD

Project : 21649



Sub-Matrix: SOIL (Matrix: SOIL)		Clie	ent sample ID	DUP 1	DUP 2	DUP 3	
,	CI	lient samplii	ng date / time	14-Dec-2017 00:00	14-Dec-2017 00:00	14-Dec-2017 00:00	
Compound	CAS Number	LOR	Unit	ES1731925-035	ES1731925-036	ES1731925-037	
,				Result	Result	Result	
EA055: Moisture Content (Dried @	0 105-110°C)						
Moisture Content		1.0	%	10.4	10.1	14.0	
EG005T: Total Metals by ICP-AES							
Arsenic	7440-38-2	5	mg/kg		10	13	
Cadmium	7440-43-9	1	mg/kg		<1	<1	
Chromium	7440-47-3	2	mg/kg		21	20	
Copper	7440-50-8	5	mg/kg		18	18	
Lead	7439-92-1	5	mg/kg		20	18	
Nickel	7440-02-0	2	mg/kg		10	9	
Zinc	7440-66-6	5	mg/kg		43	44	
EG035T: Total Recoverable Merci							
Mercury	7439-97-6	0.1	mg/kg		<0.1	<0.1	
EP068A: Organochlorine Pesticide			0 0				
alpha-BHC	319-84-6	0.05	mg/kg	<0.05	<0.05		
Hexachlorobenzene (HCB)	118-74-1	0.05	mg/kg	<0.05	<0.05		
beta-BHC	319-85-7	0.05	mg/kg	<0.05	<0.05		
gamma-BHC	58-89-9	0.05	mg/kg	<0.05	<0.05		
delta-BHC	319-86-8	0.05	mg/kg	<0.05	<0.05		
Heptachlor	76-44-8	0.05	mg/kg	<0.05	<0.05		
Aldrin	309-00-2	0.05	mg/kg	<0.05	<0.05		
Heptachlor epoxide	1024-57-3	0.05	mg/kg	<0.05	<0.05		
^ Total Chlordane (sum)		0.05	mg/kg	<0.05	<0.05		
trans-Chlordane	5103-74-2	0.05	mg/kg	<0.05	<0.05		
alpha-Endosulfan	959-98-8	0.05	mg/kg	<0.05	<0.05		
cis-Chlordane	5103-71-9	0.05	mg/kg	<0.05	<0.05		
Dieldrin	60-57-1	0.05	mg/kg	<0.05	<0.05		
4.4`-DDE	72-55-9	0.05	mg/kg	<0.05	<0.05		
Endrin	72-20-8	0.05	mg/kg	<0.05	<0.05		
beta-Endosulfan	33213-65-9	0.05	mg/kg	<0.05	<0.05		
^ Endosulfan (sum)	115-29-7	0.05	mg/kg	<0.05	<0.05		
4.4`-DDD	72-54-8	0.05	mg/kg	<0.05	<0.05		
Endrin aldehyde	7421-93-4	0.05	mg/kg	<0.05	<0.05		
Endosulfan sulfate	1031-07-8	0.05	mg/kg	<0.05	<0.05		
4.4`-DDT	50-29-3	0.2	mg/kg	<0.2	<0.2		
Endrin ketone	53494-70-5	0.05	mg/kg	<0.05	<0.05		

Page : 22 of 24 Work Order : ES1731925

Client : SMEC TESTING SERVICES PTY LTD

EP068T: Organophosphorus Pesticide Surrogate

78-48-8

0.05

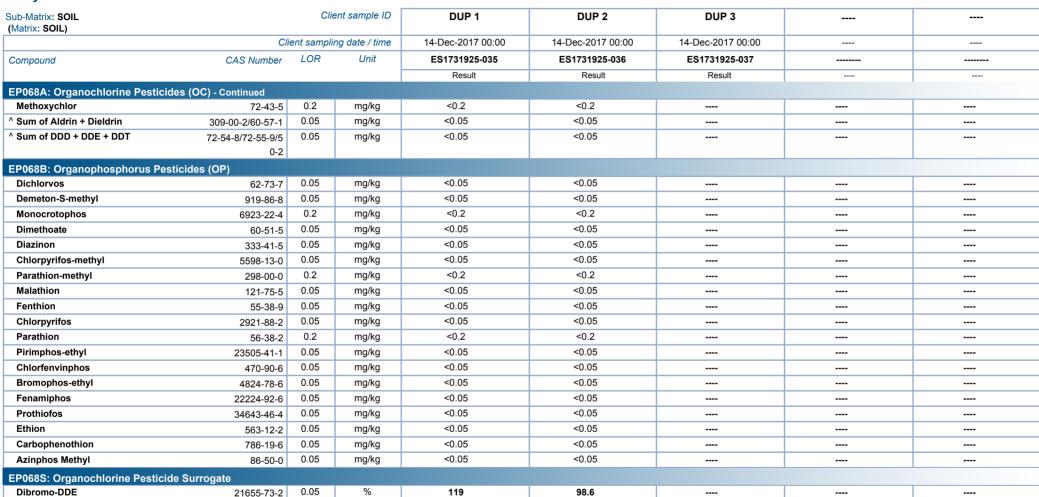
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119

DEF

Project : 21649

Analytical Results



116



Page : 23 of 24 Work Order : ES1731925

Client : SMEC TESTING SERVICES PTY LTD

Project : 21649

Analytical Results Descriptive Results

Sub-Matrix: SOIL

Method: Compound	Client sample ID - Client sampling date / time	Analytical Results
EA200: AS 4964 - 2004 Identification of Asbesto	s in Soils	
EA200: Description	21649/S8-1 - 14-Dec-2017 00:00	Mid brown clay soil.
EA200: Description	21649/S11-1 - 14-Dec-2017 00:00	Mid brown clay soil.
EA200: Description	21649/S14/1-1 - 14-Dec-2017 00:00	Mid brown clay soil.
EA200: Description	21649/S15-1 - 14-Dec-2017 00:00	Mid brown clay soil.
EA200: Description	21649/S16/1-1 - 14-Dec-2017 00:00	Mid brown clay soil.
EA200: Description	21649/S18/1-1 - 14-Dec-2017 00:00	Mid brown clay soil.



Page : 24 of 24 Work Order : ES1731925

Client : SMEC TESTING SERVICES PTY LTD

Project : 21649

Surrogate Control Limits

Sub-Matrix: SOIL		Recovery	Limits (%)
Compound	CAS Number	Low	High
EP066S: PCB Surrogate			
Decachlorobiphenyl	2051-24-3	39	149
EP068S: Organochlorine Pesticide Surrogate			
Dibromo-DDE	21655-73-2	49	147
EP068T: Organophosphorus Pesticide Surroga	ite		
DEF	78-48-8	35	143
EP075(SIM)S: Phenolic Compound Surrogates			
Phenol-d6	13127-88-3	63	123
2-Chlorophenol-D4	93951-73-6	66	122
2.4.6-Tribromophenol	118-79-6	40	138
EP075(SIM)T: PAH Surrogates			
2-Fluorobiphenyl	321-60-8	70	122
Anthracene-d10	1719-06-8	66	128
4-Terphenyl-d14	1718-51-0	65	129
EP080S: TPH(V)/BTEX Surrogates			
1.2-Dichloroethane-D4	17060-07-0	73	133
Toluene-D8	2037-26-5	74	132
4-Bromofluorobenzene	460-00-4	72	130





CERTIFICATE OF ANALYSIS

Work Order : ES1732087

Client : SMEC TESTING SERVICES PTY LTD

Contact : SMEC TESTING ALL RESULTS

Address : P O BOX 6989

WETHERILL PARK NSW, AUSTRALIA 2164

 Telephone
 : ---

 Project
 : ---

 Order number
 : ---

 C-O-C number
 : ---

 Sampler
 : ---

Site : ----

Quote number ; EN/222/17

No. of samples received : 9
No. of samples analysed : 9

Page : 1 of 5

Laboratory : Environmental Division Sydney

Contact : Customer Services ES

Address : 277-289 Woodpark Road Smithfield NSW Australia 2164

Telephone : +61-2-8784 8555

Date Samples Received : 18-Dec-2017 11:30

Date Analysis Commenced : 18-Dec-2017

Issue Date : 22-Dec-2017 17:03



This report supersedes any previous report(s) with this reference. Results apply to the sample(s) as submitted. This document shall not be reproduced, except in full.

This Certificate of Analysis contains the following information:

- General Comments
- Analytical Results

Additional information pertinent to this report will be found in the following separate attachments: Quality Control Report, QA/QC Compliance Assessment to assist with Quality Review and Sample Receipt Notification.

Signatories

This document has been electronically signed by the authorized signatories below. Electronic signing is carried out in compliance with procedures specified in 21 CFR Part 11.

Signatories	Position	Accreditation Category
Ankit Joshi	Inorganic Chemist	Sydney Inorganics, Smithfield, NSW

Celine ConceicaoSenior SpectroscopistSydney Inorganics, Smithfield, NSWEdwandy FadjarOrganic CoordinatorSydney Inorganics, Smithfield, NSW

Page : 2 of 5 Work Order : ES1732087

Client : SMEC TESTING SERVICES PTY LTD

Project · --



General Comments

The analytical procedures used by the Environmental Division have been developed from established internationally recognized procedures such as those published by the USEPA, APHA, AS and NEPM. In house developed procedures are employed in the absence of documented standards or by client request.

Where moisture determination has been performed, results are reported on a dry weight basis.

Where a reported less than (<) result is higher than the LOR, this may be due to primary sample extract/digestate dilution and/or insufficient sample for analysis.

Where the LOR of a reported result differs from standard LOR, this may be due to high moisture content, insufficient sample (reduced weight employed) or matrix interference.

When sampling time information is not provided by the client, sampling dates are shown without a time component. In these instances, the time component has been assumed by the laboratory for processing purposes.

Where a result is required to meet compliance limits the associated uncertainty must be considered. Refer to the ALS Contact for details.

Key: CAS Number = CAS registry number from database maintained by Chemical Abstracts Services. The Chemical Abstracts Service is a division of the American Chemical Society.

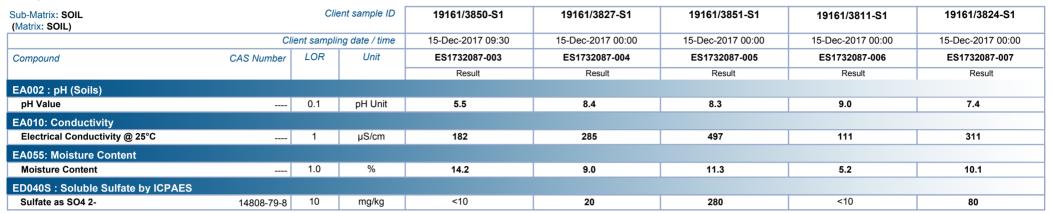
LOR = Limit of reporting

- ^ = This result is computed from individual analyte detections at or above the level of reporting
- ø = ALS is not NATA accredited for these tests.
- ~ = Indicates an estimated value.

Page : 3 of 5 Work Order : ES1732087

Client : SMEC TESTING SERVICES PTY LTD

Project : --

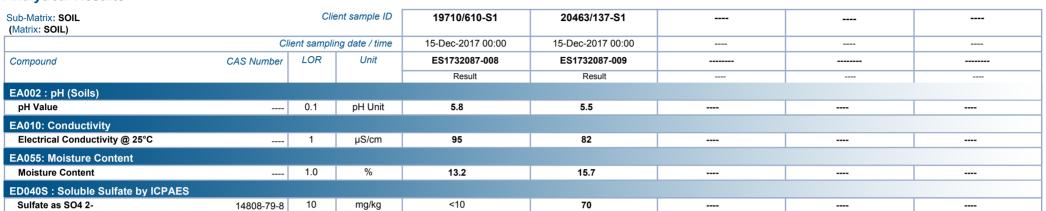




Page : 4 of 5 Work Order : ES1732087

Client : SMEC TESTING SERVICES PTY LTD

Project : ---





Page : 5 of 5 Work Order : ES1732087

Client : SMEC TESTING SERVICES PTY LTD

Project : --







APPENDIX C – BUREAU OF METEOROLOGY DATA



Climate statistics for Australian locations

Monthly climate statistics

All years of record

Note: Many statistics are updated quarterly and recent weather events may not be represented in the statistics below. For more current information on recent extreme values, please refer to the corresponding <u>Daily rainfall</u>, <u>Maximum temperature</u> and <u>Minimum temperature</u> data tables for this site, and our <u>Australian Climate and Weather Extremes Monitoring System</u>. Missing observations associated with the observer being unavailable (where observations are undertaken manually), a failure in the observing equipment, or when an event has produced suspect data may result in an extreme event not being recorded.

Site name: PROSPECT RESERVOIR

Latitude: 33.82° S

Longitude: 150.91° E

Site number: 067019

Commenced: 1887

Map

Operational status: Open

Statistics	<u>Jan</u>	Feb	Mar	Apr	May	<u>Jun</u>	<u>Jul</u>	Aug	Sep	<u>Oct</u>	Nov	Dec	Annual	Yea	<u>ars</u>
Temperature															
Maximum temperature															
Mean maximum temperature (°C)	28.5	28.0	26.4	23.7	20.4	17.4	16.9	18.8	21.5	24.0	25.6	27.5	23.2	52	1965 2017
Highest temperature (°C)	45.1	43.3	39.5	37.1	29.4	25.6	27.1	29.4	35.0	39.0	42.0	42.7	45.1	52	1965 2017
Date	18 Jan 2013	11 Feb 2017	13 Mar 1998	04 Apr 1986	10 May 1967	06 Jun 1997	30 Jul 2017	26 Aug 1995	25 Sep 1972	21 Oct 1988	20 Nov 2009	21 Dec 1994	18 Jan 2013		
Lowest maximum temperature (°C)	17.5	18.0	16.0	14.3	12.5	10.0	7.8	10.5	11.7	12.0	12.5	11.7	7.8	52	1965 2017
Date	28 Jan 1978	24 Feb 1992	09 Mar 1980	17 Apr 1983	31 May 1977	12 Jun 1975	23 Jul 1968	11 Aug 1973	05 Sep 1967	06 Oct 1978	16 Nov 1988	08 Dec 1966	23 Jul 1968		
Decile 1 maximum temperature (°C)	22.5	22.5	21.8	19.6	17.0	14.6	14.2	15.4	16.7	18.3	19.7	21.6		52	1965 2017
Decile 9 maximum temperature(°C)	35.4	34.0	31.1	27.9	23.8	20.0	19.8	22.7	27.1	31.0	32.5	34.0		52	1965 2017
Mean number of days ≥ 30 °C	11.0	8.4	5.5	0.9	0.0	0.0	0.0	0.0	1.1	3.9	5.7	9.3	45.8	52	1965 2017
Mean number of days ≥ 35 °C	3.6	2.3	0.5	0.0	0.0	0.0	0.0	0.0	0.0	0.4	1.4	2.4	10.6	52	1965 2017
Mean number of days ≥ 40 °C	0.7	0.3	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.2	0.2	1.4	52	1965 2017
Minimum temperature															
Mean minimum temperature (°C)	17.7	17.8	16.2	13.0	9.9	7.5	6.1	6.8	9.4	12.1	14.4	16.4	12.3	52	1965 2017

Statistics	<u>Jan</u>	<u>Feb</u>	Mar	Apr	May	<u>Jun</u>	<u>Jul</u>	Aug	<u>Sep</u>	Oct	Nov	<u>Dec</u>	Annual	Ye	ars
Lowest temperature (°C)	10.0	10.8	7.9	3.6	1.2	-0.8	-0.6	-0.5	1.7	4.5	6.8	7.8	-0.8	52	196 201
Date	16 Jan 1996	18 Feb 1998	30 Mar 1970	23 Apr 2006	29 May 1987	30 Jun 2010	17 Jul 2007	13 Aug 2005	01 Sep 2012	08 Oct 1998	03 Nov 2003	18 Dec 1969	30 Jun 2010		
Highest minimum temperature (°C)	26.7	26.5	23.3	21.9	17.4	15.8	16.3	17.2	19.8	24.0	24.7	25.3	26.7	52	196 201
Date	22 Jan 1967	06 Feb 2011	03 Mar 1968	05 Apr 1986	02 May 2000	10 Jun 1995	25 Jul 1990	18 Aug 1988	24 Sep 2003	03 Oct 1981	22 Nov 2006	23 Dec 2000	22 Jan 1967		
Decile 1 minimum temperature (°C)	14.6	15.0	13.0	9.6	6.0	3.9	2.7	3.5	5.7	8.3	10.6	13.0		52	196 201
Decile 9 minimum temperature (°C)	20.8	20.6	19.0	16.3	13.6	11.4	9.7	10.5	13.2	15.8	18.0	19.5		52	196 201
Mean number of days ≤ 2 °C	0.0	0.0	0.0	0.0	0.0	0.6	1.8	0.8	0.0	0.0	0.0	0.0	3.2	52	196 201
Mean number of days ≤ 0 °C	0.0	0.0	0.0	0.0	0.0	0.0	0.1	0.0	0.0	0.0	0.0	0.0	0.1	52	196 201
Ground surface temperature															
Mean daily ground minimum temperature (°C)															
Lowest ground temperature (°C)															
Date															
Mean number of days ground min. temp. ≤ -1 °C															
Statistics	Jan	Feb	Mar	Apr	May	Jun	Jul	Aug	Sep	Oct	Nov	Dec	Annual	Ye	ars
Rainfall															
Mean rainfall (mm)	95.8	96.5	98.0	76.6	69.9	77.2	55.7	50.4	46.0	58.1	72.8	75.9	875.0	129	188 201
Highest rainfall (mm)	426.7	519.1	380.7	425.0	556.0	531.3	323.7	458.5	186.3	269.0	391.3	338.1	1900.0	131	188 201
Date	1951	1956	1890	2015	1889	1950	1904	1986	1892	1916	1961	1920	1950		
Lowest rainfall (mm)	3.9	2.8	5.1	2.0	1.8	1.0	0.0	0.0	0.0	0.0	0.8	2.2	394.6	131	188 201
Date	1929	1902	1940	1997	1957	2001	1977	1995	1957	1988	1915	1979	1944		
Decile 1 rainfall (mm)	22.3	12.5	20.7	15.1	10.0	8.9	6.4	5.9	7.4	12.5	15.9	19.9	574.7	131	188 201
Decile 5 (median) rainfall (mm)	73.2	73.1	78.3	57.2	38.4	50.0	32.9	30.9	40.2	43.1	60.1	58.0	861.7	131	188 201
Decile 9 rainfall (mm)	193.7	197.7	201.7	170.5	169.9	181.0	128.1	129.6	100.5	130.7	141.7	159.4	1178.0	131	188 201
Highest daily rainfall (mm)	161.2	164.6	153.9	163.1	314.2	163.4	143.5	321.0	96.5	102.1	126.2	154.9	321.0	131	188 201
Date	31 Jan 2001	11 Feb 1956	20 Mar 1892	16 Apr 1946	28 May 1889	11 Jun 1991	10 Jul 1904	06 Aug 1986	02 Sep 1970	05 Oct 1916	14 Nov 1969	13 Dec 1910	06 Aug 1986		
Mean number of days of rain	10.7	10.7	11.0	9.4	8.9	9.5	7.8	7.9	8.4	9.2	9.6	10.0	113.1	131	188 201
Mean number of days of rain ≥ 1 mm	8.1	8.1	8.4	7.0	6.4	7.0	5.6	5.7	6.1	6.8	7.3	7.6	84.1	131	188 201
Mean number of days of rain ≥ 10 mm	2.6	2.6	2.6	2.1	1.7	2.0	1.4	1.4	1.3	1.7	2.3	2.3	24.0	131	188 201

Statistics	<u>Jan</u>	<u>Feb</u>	<u>Mar</u>	<u>Apr</u>	May	<u>Jun</u>	<u>Jul</u>	Aug	<u>Sep</u>	<u>Oct</u>	<u>Nov</u>	<u>Dec</u>	Annual	Ye	ars
Statistics	Jan	Feb	Mar	Apr	May	Jun	Jul	Aug	Sep	Oct	Nov	Dec	Annual	Ye	ars
Other daily elements															
Mean daily wind run (km)															
Maximum wind gust speed (km/h)															
Date															
Mean daily sunshine (hours)															
Mean daily solar exposure (MJ/m ²)	22.3	19.2	16.5	13.6	10.4	8.7	9.9	13.2	16.8	19.8	21.2	22.7	16.2	28	1990 2018
Mean number of clear days	6.6	5.0	6.7	8.8	9.0	10.0	11.3	13.2	11.4	8.3	6.8	7.1	104.2	33	1968 200
Mean number of cloudy days	12.6	11.7	11.7	8.0	9.5	8.3	6.6	6.3	7.1	9.2	10.6	10.5	112.1	33	1968 2004
Mean daily evaporation (mm)	5.5	4.7	3.9	2.9	2.0	1.6	1.7	2.5	3.6	4.4	5.0	5.6	3.6	44	1965 2017
Statistics	Jan	Feb	Mar	Apr	May	Jun	Jul	Aug	Sep	Oct	Nov	Dec	Annual	Ye	ars
9 am conditions															
Mean 9am temperature (°C)	21.3	21.0	19.6	16.9	13.5	10.7	9.6	11.1	14.5	17.4	18.4	20.6	16.2	42	1968 2010
Mean 9am wet-bulb temperature (°C)	18.5	18.6	17.3	14.7	11.8	9.0	7.7	8.7	11.3	13.7	15.3	17.2	13.6	39	1968 2010
Mean 9am dew-point temperature (°C)	16.4	17.0	15.6	12.6	10.0	7.0	5.3	5.6	7.8	10.1	12.5	14.5	11.2	37	1974 2010
Mean 9am relative humidity (%)	75	79	79	77	80	79	76	70	65	65	70	70	74	37	1974 2010
Mean 9am cloud cover (oktas)	4.8	4.9	4.5	3.7	3.8	3.6	3.2	2.9	3.2	4.0	4.4	4.5	4.0	45	1965 2010
Mean 9am wind speed (km/h)	7.5	7.0	7.3	8.0	7.7	8.0	8.1	9.2	9.6	10.0	8.5	8.2	8.3	44	1965 2010
Statistics	Jan	Feb	Mar	Apr	May	Jun	Jul	Aug	Sep	Oct	Nov	Dec	Annual	Ye	ars
3 pm conditions															
Mean 3pm temperature (°C)	26.8	26.3	24.8	22.4	19.2	16.5	15.9	17.4	19.6	22.1	23.4	25.9	21.7	33	1968 2001
Mean 3pm wet-bulb temperature (°C)	20.0	20.0	18.8	16.4	14.4	12.0	10.8	11.5	13.2	15.3	16.9	18.8	15.7	31	1968 200
Mean 3pm dew-point temperature (°C)	15.3	15.7	14.4	11.3	9.9	6.9	4.8	4.5	6.3	8.8	11.5	13.5	10.2	28	1974 2001
Mean 3pm relative humidity (%)	52	54	55	52	57	55	50	45	45	46	50	49	51	28	1974 2001
Mean 3pm cloud cover (oktas)	4.8	5.0	4.8	4.2	4.3	4.2	3.9	3.8	3.9	4.4	4.8	4.6	4.4	33	1968 2001
Mean 3pm wind speed (km/h)	12.7	12.4	12.0	11.5	10.3	12.3	12.4	14.3	15.3	15.4	14.4	14.5	13.1	30	1968 2001

red = highest value blue = lowest value

Product IDCJCM0037 Prepared at Thu 11 Jan 2018 02:39:03 AM EST

Monthly statistics are only included if there are more than 10 years of data. The number of years (provided in the 2nd last column of the table) may differ between elements if the observing program at the site changed. More detailed data for individual sites can be obtained by contacting the Bureau.

Related Links

- This page URL: http://www.bom.gov.au/climate/averages/tables/cw_067019_All.shtml
- Summary statistics and locational map for this site: http://www.bom.gov.au/climate/averages/tables/cw 067019.shtml
- About climate averages: http://www.bom.gov.au/climate/cdo/about/about-stats.shtml
- Data file (csv): http://www.bom.gov.au/clim_data/cdio/tables/text/IDCJCM0037_067019.csv
- Climate averages home page URL: http://www.bom.gov.au/climate/data/index.shtml
- Bureau of Meteorology website: http://www.bom.gov.au

Page created: Thu 11 Jan 2018 02:39:03 AM EST

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APPENDIX D -WATER BALANCE CALCULATIONS

MONTHLY WATER BALANCE USED TO DETERMINE WET WEATHER STORAGE

Design Wastewater Flow	Q	l/day	1080
Design Percolation Rate	R	mm/wk	21
Land Area	L	m ²	475

Paramters	Symbol	Formula	Units	Jan	Feb	Mar	Apr	May	Jun	Jul	Aug	Sep	Oct	Nov	Dec	Total
Days in Month	D	-	days	31	28	31	30	31	30	31	31	30	31	30	31	365
Precipitation	Р	-	mm/month	73.2	73.1	78.3	57.2	38.4	50	32.9	30.9	40.2	43.1	60.1	58	635.4
Evaporation	E	-	mm/month	170.5	131.6	120.9	87	62	48	52.7	77.5	108	136.4	150	173.6	1318.2
Crop Factor	С	-	-	0.6	0.6	0.6	0.6	0.6	0.6	0.6	0.6	0.6	0.6	0.6	0.6	-

Inputs																
Precipitation	Р	-	mm/month	73.2	73.1	78.3	57.2	38.4	50	32.9	30.9	40.2	43.1	60.1	58	635.4
Effluent Irrigation	W	(Q x D) / L	mm/month	70.5	63.7	70.5	68.2	70.5	68.2	70.5	70.5	68.2	70.5	68.2	70.5	829.9
Inputs		P + W	mm/month	143.7	136.8	148.8	125.4	108.9	118.2	103.4	101.4	108.4	113.6	128.3	128.5	1465.3

Outputs																
Evapotranspiration	ET	ExC	mm/month	102.30	78.96	72.54	52.20	37.20	28.80	31.62	46.50	64.80	81.84	90.00	104.16	790.92
Percolation	В	(R / 7) x D	mm/month	93.0	84.0	93.0	90.0	93.0	90.0	93.0	93.0	90.0	93.0	90.0	93.0	1095.0
Outputs		ET + B	mm/month	195.3	163.0	165.5	142.2	130.2	118.8	124.6	139.5	154.8	174.8	180.0	197.2	1885.9
Storage	S	(P + W) - (ET + B)	mm/month	-51.6	-26.2	-16.8	-16.8	-21.3	-0.6	-21.2	-38.1	-46.4	-61.3	-51.7	-68.7	-